<u>PROJ</u>	ECT_DESCRIPTION	BASI	<u>IS OF DESIG</u>
1.	THE STRUCTURAL WORK SHOWN ON THESE DRAWINGS IS FOR THE CONSTRUCTION OF A NEW ONE-STORY RESTROOM BUILDING AND RAMP.	1.	ALL NEW WHICH C AMENDME
2.	THE DESIGN OF NEW WORK IS IN GENERAL CONFORMANCE TO THE REQUIREMENTS OF THE 2010 SAN FRANCISCO BUILDING CODE (SFBC) WHICH COMPRISES THE 2010 CALIFORNIA BUILDING CODE (CBC) AND 2010 SAN FRANCISCO AMENDMENTS.	2.	THE PUB THE CBC
<u>GENE</u>	RAL		CONFLICT
1.	THESE GENERAL NOTES APPLY THROUGHOUT ALL STRUCTURAL DRAWINGS EXCEPT WHERE SPECIFICALLY SHOWN BY NOTES ON DRAWINGS AND/OR DETAILS.		ACI 30
2.	THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO THE START OF CONSTRUCTION OR FABRICATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION OR FABRICATION. ANY DISCREPANCIES, INCONSISTENCIES, OR UNSOUND CONDITIONS SHALL BE REPORTED TO THE ENGINEER FOR RESOLUTION PRIOR TO THE START OF		ACI 31 ASCE
	ANY CONSTRUCTION OR FABRICATION SO THAT A CLARIFICATION CAN BE ISSUED.		ASTM
3.	DIMENSIONS ARE TO CENTERLINE OF STEEL FRAMING, FACE OF CONCRETE SURFACES, FACE OF STUDS, FACE OF CONCRETE MASONRY UNITS (CMU), TOP OF SHEATHING, OR TOP OF STRUCTURAL SLAB, UNLESS OTHERWISE NOTED.		AWS D
4.	DIMENSIONS IN THE STRUCTURAL DRAWINGS ARE AS NOTED. DO NOT USE DIMENSIONS SCALED FROM THE STRUCTURAL DRAWINGS.	STRU	JCTURAL DES
5.	ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL	1.	DESIGN L
	BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE CITY AND COUNTY OF SAN FRANCISCO.		ROOF:
ô.	ALL TYPICAL DETAILS AND NOTES SHOWN ON DRAWINGS SHALL APPLY UNLESS OTHERWISE NOTED.	2.	WIND DES
	TYPICAL DETAILS MAY NOT NECESSARILY BE INDICATED ON THE PLANS, BUT SHALL STILL APPLY AS SHOWN OR DESCRIBED IN THE DETAILS. WHERE TYPICAL DETAILS ARE NOTED ON THE		BASIC WI
	DRAWINGS, THE SPECIFIED TYPICAL DETAIL SHALL BE USED. WHERE NO DETAILS ARE NOTED, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. THE CONTRACTOR SHALL SUBMIT ALL PROPOSED ALTERNATE TYPICAL DETAILS TO THOSE PROVIDED WITH RELATED CALCULATIONS TO THE	3.	SEISMIC [
	ENGINEER FOR APPROVAL PRIOR TO SHOP DRAWING PRODUCTION AND FIELD USE.		PEAK GRO SITE CLAS
7.	REFER TO OTHER DISCIPLINES' DRAWINGS AND COORDINATE INFORMATION RELATED TO THOSE OTHER DISCIPLINES' SYSTEMS FOR ITEMS SUCH AS:		SITE COEI MAXIMUM
	a. FINISH FLOOR ELEVATIONS, FLOOR DEPRESSIONS, CHANGES IN ELEVATION, SLOPES, DRAINS, CURBS, PADS, INSERTS, ETC.		DESIGN S
	 b. SIZE AND LOCATION OF ALL NON-BEARING PARTITIONS. c. SIZE AND LOCATION OF ALL WALL, FLOOR AND ROOF OPENINGS. 		IMPORTAN SEISMIC [
	 d. STAIR FRAMING, HANGERS AND DETAILS. e. WATERPROOFING AND WATERSTOPS. 		FOR BEAF
	f. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND FLOOR OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.		RESPONSE SYSTEM (
	 g. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS, ETC., IN WALLS AND SLABS. h. SIZE, LOCATION, ANCHORAGE AND BRACING FOR MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT. 		DEFLECTIC SEISMIC E
8.	FOR OPENINGS LARGER THAN 6"THAT ARE REQUIRED BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT DRAWINGS INDICATING OPENING LOCATIONS TO THE ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.		
9.	THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS AND/OR METHODS OF CONSTRUCTION. ALTHOUGH THE NEED FOR SHORING MAY SOMETIMES BE INDICATED IN THE STRUCTURAL DRAWINGS, IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DESIGN, PROVIDE, AND MAINTAIN TEMPORARY BRACING, SHORING, GUYING, OR OTHER TEMPORARY SUPPORT AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION.		
10.	THE CONTRACTOR SHALL PROVIDE ALL NECESSARY PROTECTION OF ADJACENT STRUCTURES DURING CONSTRUCTION. THE CONTRACTOR SHALL BEAR ALL EXPENSE FOR REPAIR OR REPLACEMENT.		
11. ;	WALLS SHALL BE ADEQUATELY BRACED DURING CONSTRUCTION UNTIL WALL DESIGN STRENGTH HAS BEEN ATTAINED AND ALL PERMANENT SUPPORTS ARE IN PLACE.		
12.	THE USE OF NEW CONSTRUCTION FOR TEMPORARY SUPPORT OR STORAGE OF CONSTRUCTION EQUIPMENT OR MATERIALS IS RESTRICTED TO THE DESIGN CAPACITY OF THE NEW CONSTRUCTION AT THE TIME IT IS TO BE USED. EQUIPMENT OR MATERIALS SHALL BE PLACED SO AS NOT TO EXCEED THE CAPACITY OF INDIVIDUAL ELEMENTS. PROVIDE ADEQUATE, ENGINEERED SHORING AND/OR BRACING WHERE DESIGN CAPACITY IS NOT SUFFICIENT.		
13.	CONSTRUCTION LOADS SHALL NOT BE PLACED ON NEW CONCRETE CONSTRUCTION, INCLUDING CONCRETE FILL ON METAL DECK, FOR AT LEAST 7 DAYS AFTER CONCRETE PLACEMENT.		
4.	SPECIFICATIONS AND DETAILING OF ALL WATERPROOFING AND DRAINAGE ITEMS, ALTHOUGH SOMETIMES INDICATED ON THE STRUCTURAL DRAWINGS FOR GENERAL INFORMATION PURPOSES ONLY, ARE SOLELY THE DESIGN RESPONSIBILITY OF OTHERS.		
5.	IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING PIPES, DUCTS, AND UTILITIES, WHETHER SHOWN HEREIN OR NOT, AND TO PROTECT THEM FROM DAMAGE. THE CONTRACTOR SHALL BEAR ALL EXPENSE FOR REPAIR OR REPLACEMENT.		
6.	ALL STRUCTURAL MEMBERS AND ELEMENTS SHOWN ON THE STRUCTURAL DRAWINGS ARE NEW UNLESS NOTED (E) FOR EXISTING CONDITIONS.		

W CONSTRUCTION SHALL CONFORM TO THE 2010 SAN FRANCISCO BUILDING CODE (SFBC) COMPRISES THE 2010 CALIFORNIA BUILDING CODE (CBC) AND 2010 SAN FRANCISCO MENTS.

BLICATIONS LISTED BELOW ARE THE GOVERNING CODES AND STANDARDS REFERENCE BY C AND ARE REFERENCED HEREIN BY THEIR BASIC DESIGNATION. IN THE CASE OF CTING REQUIREMENTS, THE SFBC SHALL GOVERN.

CI 301-10	AMERICAN CONCRETE INSTITUTE,	"SPECIFICATIONS	FOR STRUCTURAL
	CONCRETE", 2010 EDITION		

318–08 AMERICAN CONCRETE INSTITUTE, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", 2005 EDITION

E 7-10 AMERICAN SOCIETY OF CIVIL ENGINEERS, "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES", 2005 EDITION

AMERICAN SOCIETY FOR TESTING AND MATERIALS

D1.4 AMERICAN WELDING SOCIETY, "STRUCTURAL WELDING CODE – REINFORCING STEEL", 2000 EDITION

ESIGN CRITERIA

LIVE LOADS:

F:	20 PSF (REDUCIBLE)
DESIGN CRITERIA:	
C WIND SPEED: DSURE:	85 MPH C
MIC DESIGN CRITERIA:	
COEFFICIENTS: MUM SPECTRAL ACCELERATION: GN SPECTRAL ACCELERATION: JPANCY CATEGORY:	$F_0 = 1.00, FV = 1.50$ $S_{MS} = 1.730g, S_{M1} = 1.324g$ $S_{DS} = 1.153g, S_{D1} = 0.883g$

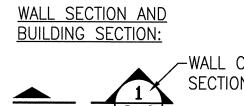
ARING WALL SYSTEMS WITH ORDINARY REINFORCED CONCRETE SHEAR WALLS:

E MODIFICATION FACTOR: OVERSTRENGTH FACTOR: TION AMPLIFICATION FACTOR: BASE SHEAR:

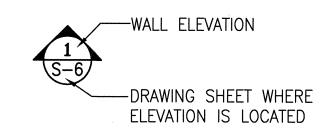
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					DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer
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S1.1	STRUCTURAL GENERAL NOTES	X X	X X		
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S1.3 S1.4	STRUCTURAL GENERAL NOTES TYPICAL CONCRETE DETAILS	X X	X X		C - C - C - C - C - C - C - C - C - C -
S1.5	TYPICAL CONCRETE DETAILS	× X	X		45.05
S2.0	SITE PLAN	X	X		Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager
S2.1 S3.1	FOUNDATION AND ROOF PLANS FOUNDATION DETAILS	X	X X		30 Van Ness Avenue Suite 4100 San Francisco, CA (415) 557-4700 94102-6028 Fax (415) 5574701
S4.1	WALL ELEVATIONS	Х	Х		
S4.2 S5.1	WALL SECTIONS & DETAILS ROOF DETAILS	X	X		Project Providence Pro
S6.1	RETAINING WALLS PLAN & ELEVATION	X	X		2008 PARK BOND RESTROOM
S6.2	RETAINING WALLS DETAILS	Х	X		REPLACEMENT PROJECT CONTEMPORARY DESIGN
S6.3 S6.4	RETAINING WALLS DETAILS RETAINING WALLS DETAILS	X X	X X		DUPONT TENNIS COURTS RESTROOMS
					336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007
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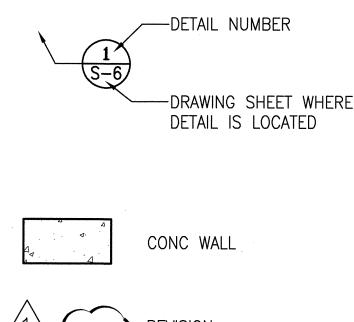
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<u>DETAIL REF</u>



FOUND/	ATIONS	CON	CRETE
	THE FOUNDATION DESIGN IS BASED UPON THE PROJECT GEOTECHNICAL REPORT "GEOTECHNICAL MEMORANDUM – BATHROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN" PREPARED BY THE DEPARTMENT OF PUBLIC WORKS BUREAU OF ENGINEERING, DATED 11/15/2011.	1.	MIXING, BATCHING, TRANSPORTING, PLACING, AND CURING OF ALL CONCRETE AND SPECIFICATION OF CONCRETE MATERIALS, SHALL CONFORM TO ACI 301 "SPECIFICATION FOR STRUCTURAL CONCRETE", EXCEPT AS NOTED BELOW.
	DESIGN SOIL PARAMETERS: ALLOWABLE BEARING PRESSURE: 2,500 PSF FOR DEAD LOADS + LIVE LOADS 3,500 PSF FOR DEAD LOADS + LIVE LOADS + LATERAL LOADS	2.	CONCRETE SHALL BE READY—MIXED CONFORMING TO ASTM C94. CEMENT SHALL BE PORTLAND CEMENT TYPE II, CONFORMING TO ASTM C150. ALL CONCRETE USED IN SUSPENDED SLABS AN SLABS—ON—GRADE SHALL BE DESIGNED WITH A SHRINKAGE LIMITATION OF 0.04% AFTER 28 DAY OF DRYING.
	REFER TO THE GEOTECHNICAL MEMORANDUM FOR ADDITIONAL INFORMATION AND RECOMMENDATIONS NOT NOTED HERE.	3.	CONCRETE MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER AND APPROVED PRIOR TO USI SELECTION OF CONCRETE MIX PROPORTIONS SHALL BE IN ACCORDANCE WITH ACI 301. MIX PROPORTIONS SHALL MEET OR EXCEED THE REQUIREMENTS LISTED BELOW FOR THE LOCATIONS
	THE GEOTECHNICAL ENGINEER SHALL VERIFY THE CONDITIONS AND/OR ADEQUACY OF ALL SUBGRADES, ENGINEERED FILLS, AND BACKFILLS BEFORE PLACEMENT OF FILLS, FOOTINGS, SLABS, OR OTHER CONSTRUCTION DEPENDENT UPON THEM.	4.	NOTED. THE MORE STRINGENT OF THE REQUIREMENTS LISTED SHALL GOVERN. SUPPLEMENTARY CEMENTITIOUS MATERIALS (SCM), SUCH AS SLAG, FLY ASH, SILICA FUME, AND
	EXCAVATIONS FOR FOOTINGS SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING REINFORCING AND CONCRETE. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER WHEN THE EXCAVATIONS ARE READY FOR OBSERVATION BY THE GEOTECHNICAL ENGINEER.		CALCINED CLAY, AS A PERCENTAGE OF TOTAL WEIGHT OF CEMENTITIOUS MATERIAL SHALL BE A MINIMUM OF 25 PERCENT AND A MAXIMUM OF 50 PERCENT. COAL FLY ASH, AS A PERCENTAG OF TOTAL WEIGHT OF CEMENTITIOUS MATERIAL, SHALL BE A MAXIMUM OF 20 PERCENT. COAL FLY ASH SHALL BE CLASS F, MEETING ASTM C618 REQUIREMENTS. FINELY GROUND GRANULAT BLAST-FURNACE SLAG SHALL CONFORM TO ASTM C989. WATER/CEMENT RATIO SHALL BE BASE
	FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED NATIVE SOIL OR ENGINEERED FILL. ALL ABANDONED FOOTINGS, UTILITIES, ETC., SHALL BE REMOVED. ALL FOOTINGS SHALL BE FOUNDED AT A DEPTH AT LEAST 30" BELOW THE LOWEST ADJACENT GRADE. FOOTING DEPTHS SHOWN ON THE STRUCTURAL DRAWINGS ARE MINIMUM DEPTHS AND SHALL BE VERIFIED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.	5.	ON TOTAL CEMENTITIOUS MATERIAL, INCLUDING SUPPLEMENTARY CEMENTITIOUS MATERIALS. PROPORTIONS OF AGGREGATE TO CEMENTITIOUS PASTE SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX THAT CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFAC WATER. SUPERPLASTICIZERS MAY BE USED TO IMPROVE WORKABILITY IN THIN OR CONGESTED SECTIONS.
	SIDES OF FOUNDATIONS SHOWN STRAIGHT ARE FORMED. IF SITE CONDITIONS ALLOW AND GEOTECHNICAL ENGINEER CONCURS, SIDES OF FOUNDATION MAY BE FORMED OR NOT FORMED AT CONTRACTOR'S OPTION.	6.	CONCRETE SHALL HAVE THE FOLLOWING CHARACTERISTICS:
3.	WHERE FOUNDATIONS ARE CAST AGAINST EARTH, SLOPE SIDES OF EXCAVATIONS AS APPROVED BY GEOTECHNICAL ENGINEER. CONTRACTOR SHALL BE RESPONSIBLE FOR CLEAN-UP OF SLOUGHED MATERIALS BEFORE AND DURING CONCRETE PLACEMENT. CONCRETE COVER FOR REINFORCEMENT MAY BE AFFECTED.		MAXIMUMMAXIMUMSTRENGTH,TESTAGGREGATEWATER/CEMENTMAXLOCATIONfc' MINAGESIZERATIOSLUMPMAT4,000 PSI28 DAYS34"0.454"COLUMNS4,000 PSI28 DAYS34"0.453 1/2"
	ENGINEERED FILL BELOW BUILDING FOOTINGS SHALL BE COMPACTED TO 95% RELATIVE COMPACTION AS DETERMINED BY THE ASTM D1557 COMPACTION TEST METHOD AND UNDER THE OBSERVATION OF THE GEOTECHNICAL ENGINEER. ENGINEERED FILL SHALL EXTEND AT LEAST 5 FEET BEYOND THE BUILDING PERIMETER.	7.	WALLS4,000 PSI28 DAYS34"0.453 ½"SUSPENDED SLABS4,000 PSI28 DAYS34"0.453 ½"PIPES OTHER THAN ELECTRICAL CONDUITS SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE
	CONTRACTOR SHALL PROVIDE FOR DE-WATERING IF WATER IS PRESENT IN THE EXCAVATION. DE-WATERING PLANS SHALL BE SUBMITTED FOR REVIEW. DE-WATERING PLANS MAY INCLUDE A MONITORING PROGRAM TO EVALUATE SETTLEMENT IN THE ADJACENT IMPROVEMENTS. SEE GEOTECHNICAL REPORT.		EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. OUTSIDE DIAMETER OF CONDUIT EMBEDDED IN CONCRETE SHALL NOT EXCEED 1/6 TIMES THE MEMBER THICKNESS, OR 1 ¼", WHICHEVER IS LESS, WITHOUT APPROVAL OF THE ENGINEER. MINIMUM CLEAR DISTANCE BETWEE CONDUITS OR REBAR SHALL BE 3 TIMES CONDUIT—DIAMETER (LARGER CONDUIT) OR 1 INCH, WHICHEVER IS GREATER. CONDUIT EMBEDDED IN SLABS SHALL BE EMBEDDED IN ONE LAYER A MID—DEPTH OF SLABS. CONDUITS SHALL BE FIRMLY CHAIRED AND TIED TO PREVENT
	ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE THE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH UNLESS SPECIFICALLY APPROVED BY THE ENGINEER IN WRITING. THE CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL STRENGTH. THE CONTRACTOR SHALL PROVIDE FOR THE DESIGN, PERMITS, AND INSTALLATION OF SUCH BRACING.		DISPLACEMENT DURING CONCRETE PLACEMENT. CONDUIT CAN BE TIED TO REBAR WHEN ORIENT PERPENDICULAR TO THEM, PROVIDE THE LOCATION OF THE REBAR IS NOT AFFECTED BY THE CONDUIT. PLACE #3 AT 12 INCHES ADDED REINFORCEMENT PERPENDICULAR TO CONDUITS WHERE REQUIRED TO SUPPORT CONDUIT. CONDUITS WITHOUT CLEARANCE NOTED ABOVE SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO INSTALLATION. ADDED TRIM REINFORCEMENT WILL BE REQUIRED WHERE CLEARANCES CANNOT BE MET, SUCH AS ELECTRICAL PANEL ROOMS.
	OVER-EXCAVATED FOOTINGS SHALL BE BACKFILLED WITH CONTROLLED LOW STRENGTH MATERIAL (CLSM) (fc'min = 100 PSI, fc'max = 1,200 PSI).	8.	SLEEVES, WHEN EMBEDDED IN CONCRETE, SHALL BE SPACED WITH ONE SLEEVE-DIAMETER (LARGER SLEEVE) CLEAR BETWEEN ADJACENT SLEEVES OR REBAR, OR 1 INCH, WHICHEVER IS
	THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF APPROPRIATE, ADEQUATE SHORING AND BRACING OF FOUNDATION EXCAVATION, AND UNDERPINNING OF EXISTING STRUCTURES TO ENSURE PROTECTION OF LIFE AND ADJACENT PROPERTY, STRUCTURES, STREETS,		GREATER. SLEEVES WITHOUT CLEARANCE NOTED ABOVE SHALL BE SUBMITTED TO THE ARCHITEC FOR REVIEW PRIOR TO INSTALLATION. ADDED TRIM REINFORCEMENT WILL BE REQUIRED WHERE CLEARANCES CANNOT BE MET, SUCH AS ELECTRICAL PANEL ROOMS.
	AND UTILITIES IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES. UNDERPINNING, SHORING, LAGGING, ETC., SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA AND SHALL BE CONSTRUCTED UNDER SEPARATE PERMIT. SHORING PLAN TO BE SUBMITTED TO THE GEOTECHNICAL ENGINEER AND THE STRUCTURAL ENGINEER	9.	ALUMINUM PIPES, CONDUITS, AND SLEEVES SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE.
	FOR REVIEW TO ENSURE CONFORMANCE WITH DESIGN DOCUMENTS. THE CONTRACTOR SHALL NOT UNDERMINE EXISTING FOUNDATIONS AND STRUCTURES DURING	10.	THE CONTRACTOR SHALL INFORM THE ENGINEER AT LEAST 3 DAYS PRIOR TO POURING ANY STRUCTURAL CONCRETE SO THAT THE ENGINEER MAY HAVE THE OPPORTUNITY OF REVIEWING TH WORK PRIOR TO CONCRETE PLACEMENT.
	EXCAVATION. IF UNDERMINING OCCURS, THE CONTRACTOR SHALL PROVIDE CORRECTIVE MEASURES FOR ENGINEER TO REVIEW AND APPROVE AT CONTRACTOR'S EXPENSE.	11.	ALL CONCRETE EXCEPT SLABS-ON-GRADE 6"THICK OR LESS SHALL BE MECHANICALLY VIBRATE AS TO COMPLETELY FILL THE FORM WITHOUT CAUSING UNDUE SEGREGATION.
	THE GEOTECHNICAL ENGINEER SHALL PREPARE A LETTER FOR THE DEPARTMENT OF BUILDING INSPECTION GIVING AN OPINION REGARDING CONFORMANCE OF THE FOOTING EXCAVATIONS, ENGINEERED FILL COMPACTION, SUBGRADE PREPARATION, AND BACKFILL WITH THE REQUIREMENTS CONTAINED IN THE GEOTECHNICAL REPORT.	12.	FOR EACH CLASS OF CONCRETE, FOUR TEST CYLINDERS FROM EACH 150 CUBIC YARDS OR 5,0 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS, PLACED IN ANY ONE DAY, SHALL BE SECURED AND TESTED BY THE BUREAU OF CONSTRUCTION MANAGEMENT – ONE TO BE TESTED AT 7 DAYS, TWO AT 28 DAYS, AND THE FOURTH HELD IN RESERVE. FOR POST-TENSIONED CONCRETE, SECURE FIVE CYLINDERS PER 150 CUBIC YARDS OR 5,000 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS, PLACE IN ANY ONE DAY, TWO SETS MINIMUM – ONE TO BE TESTED AT 4 DAYS, TWO AT 28 DAYS, AND TWO HELD IN RESERVE.

- 13. THE CONTRACTOR SHALL REMOVE AND REPLACE ANY CONCRETE WHICH FAILS TO ATTAIN SPECIFIED STRENGTH IN 28 DAYS IF SO DIRECTED BY THE ENGINEER. ANY DEFECTS IN THE HARDENED CONCRETE SHALL BE SATISFACTORILY REPAIRED OR THE HARDENED CONCRETE SHALL BE REPLACED.
- 14. ALL CONSTRUCTION JOINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 318 AND THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS, OR OTHER FOREIGN MATTER PRIOR TO PLACING THE ADJACENT CONCRETE. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ARCHITECT FOR REVIEW PRIOR TO START OF CONSTRUCTION.
- 15. WHERE NEW CONCRETE IS TO BE CAST AGAINST EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE ROUGHENED TO A MINIMUM OF 1/4" AMPLITUDE BY SANDBLASTING OR BUSH HAMMERING. THE EXISTING SURFACE SHALL BE CLEANED AND LAITANCE REMOVED. APPLY "SIKADUR 32, HI-MOD" EPOXY BONDING ADHESIVE, AS MANUFACTURED BY SIKA CORPORATION, LYNDHURST, NEW JERSEY, OR APPROVED EQUAL, TO EXISTING CONCRETE SURFACE PRIOR TO PLACEMENT OF NEW CONCRETE.

REINFORCING STEEL

- REINFORCING STEEL DETAILING, FABRICATION, AND PLACEMENT SHALL CONFORM TO THE ACI 318, CHAPTER 7.
- REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

DEFORMED BARS DEFORMED BARS USED IN SHEAR WALLS AND MOMENT-RESISTING-FRAMES WELDED REINFORCEMENT, WHEN SPECIFIED BY THE EN WELDED WIRE FABRIC (WWF) (SMOOTH WIRE) WELDED WIRE REINFORCEMENT (DEFORMED WIRE)

- ALL STEEL REINFORCING BAR BENDS SHALL BE MADE COLD.
- REINFORCEMENT AND EMBEDMENTS SHALL BE ACCURATELY POSITIONED AND SECURED AGAINST DISPLACEMENT BEFORE AND DURING CONCRETE PLACEMENT. PROVIDE SUFFICIENT SUPPORTS TO PREVENT DAMAGE OR DISPLACEMENT DUE TO CONSTRUCTION TRAFFIC ON REINFORCEMENT.
- PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. SPLICE ONLY AS SHOWN OR 5 APPROVED.
- WHERE NOTED ON PLANS, PROVIDE THREADED COUPLERS CAPABLE OF DEVELOPING 125% OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCING STEEL. THREADED COUPLERS SHALL BE "LENTON COUPLERS", AS MANUFACTURED BY ERICO COMPANY, SOLON, OHIO, OR APPROVED EQUAL WITH CURRENT ICC-ES EVALUATION REPORT.
- 7. WELDING (INCLUDING TACK WELD) OR REINFORCING BARS IS PROHIBITED EXCEPT WHERE DETAILED OR APPROVED IN WRITING BY ENGINEER.
- REINFORCEMENT CROSSING CONSTRUCTION JOINTS SHALL BE CONTINUOUS OR LAP SPLICED PER TENSION LAP TABLE OR APPROVED COUPLERS.
- 9. MINIMUM CLEAR COVER DISTANCES FROM FINISHED FACE OF CONCRETE TO STEEL REINFORCEMENT SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"

CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS #5 BAR, W31 OR D31 WIRE, AND SMALLER

CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOISTS #14 AND #18 BARS #11 BAR AND SMALLER

BEAMS, COLUMNS

10. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL INCLUDE ELEVATION OF ALL BEAMS AND COLUMNS SHOWING BAR AND LAP LOCATIONS. SUBMIT MILL CERTIFICATES FOR REINFORCING STEEL PRIOR TO REBAR PLACEMENT.

ASTM A615 OR ASTM A706, GRADE 60

ENGINEER		•	GRADE 60 GRADE 60
	ASTM		
	ASTM	A496,	ASTM A497





DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer

BUILDING DESIGN & CONSTRUCTION



Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager 30 Van Ness Avenue Suite 4100 San Francisco, CA (415) 557-4700 94102-6028 Fax (415) 5574701

Project

2008 PARK BOND RESTROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN

DUPONT TENNIS COURTS RESTROOMS 336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007

Consultant

DEPARTMENT OF PL CITY & COUNTY OF S	INFRASTRUCTURE DIVISION DEPARTMENT OF PUBLIC WORKS CITY & COUNTY OF SAN FRANCISCO					
30 VAN NESS AVENL SAN FRANCISCO, C						
DESIGNED BY: 5L	SL	DATE 09/2012				
DRAWN BY: BH	BH	09/2012				
CHECKED BY: JC-	JC	09/2012				
APPROVED						
SECTION MANAGER		0/9/12 0/1. DATE:				
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DIVISION MANAGER:		DATE:				

BID & CONSTRUCTION

No.	Date	Revisions
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Section Head T. LEUNG	PROFESSIONAL
Proj. Mgr. M. YEE	AND R. LE
Proj. Arch. T. LEUNG	No. S 4094 F
Drawn	*
Date SEPTEMBER 2012	PUCTURA INT
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Drawing Title

STRUCTURAL GENERAL NOTES

Sheet No.

S1.2

NONE

Scale

Job No.

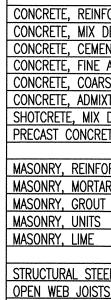
1 INTRODUCTION INTRODUCTION INTRODUCTION INTRODUCTION 1 INTRODUCTION INTRODUCTION INTRODUCTION STRUCTION 1 ADDESIVE AND MECHANICAL ANCHORS STRUCTION STRUCTION STRUCTION 1 ADDESIVE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT-RE 500-SD EPOXY" IMMEDIAI 1 ADDESIVE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT-RE 500-SD EPOXY" IMMEDIAI 1 ADDESIVE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT-RE 500-SD EPOXY" IMMEDIAI 1 ADDESIVE ANCHORS IN SA MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR APPROVED EQUAL, AND HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRUCTURE 2 ADHESIVE ANCHORS IN CONCRETE AND MANERY SHALL BE INSTALLEO WITH THE FOLLOWING MINIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD MAD/OR TORQUE TEST LOAD, U.O.N.: I. COMPANY 1 COMPERTION ADM SMY 2, 000 POUNDS 50 FOOT-POUNDS 2. SHOT 1 JOIA 3% 2,000 POUNDS 50 FOOT-POUNDS 2. SHOT 2 SHOT DIA 4% 9,000 POUNDS 50 FOOT-POUNDS 2. SHOT 1 JOIA 3% 2,000 POUNDS 50 FOOT-POUNDS 2. SHOT 3. OTHER SHOLE ANCHORE DOLT 3. OTHER SHOLE ANCHORE D					
1. NON-SHRINK GROUT SHALL BE "SIKAGROUT 212", AS MANUFACTURED BY SIKA CORPORATION, LYMDHURST, NW JERSEY, OR APPROVED EQUAL. NON-SHRINK GROUT SHALL BE NON-METALLIC AND CONTAIN NO CHLORIDES. OCHTINE WITH H MORE'S 2. ADHESINE AND CHLORIDES FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT-RE 500-SD EPDXY" (ICC-ES ESR-2322), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON SET-XP" (ICC-ES ESR-2322), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR APPROVED EQUAL. ADHESINE ANCHORS SHALL CONFORM TO ICC-ES ACEPTANCE CRITERIA ACOUST FOR CRACKED AND UNCHACKED CONCRETE. STRUCTRE COMPANY, INC., THE CONFORMENCE CRITERIA ACOUST FOR CRACKED AND UNCHACKED CONCRETE. 2. ADHESINE ANCHORS IN CONCRETE AND MAONSRY SHALL BE INSTALLED WITH THE FOLLOWING MINIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD AND/OR TORQUE TEST LOAD, U.O.N.: TO COMPANY, INC., THE COMPANY, INC., THE ADDE AND UNCHACKED CONCRETE. 2. ADHESINE ANCHORS IN CONCRETE AND MAONSRY SHALL BE INSTALLED WITH THE FOLLOWING MINIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD MIN TORQUE #G OR ½ DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 2.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 5½" CAOO POUNDS 50 FOOT-POUNDS 1.50F #G OR ½" DIA 51" CLCC-ES ESR-1917, AS MANUFACTURED BY HILT INC., TULSA, CONTRACTOR'S EXPENSE. 3. THE ENT STRUCTURE 3. THE ENT MINICALE 3. THE ENT MINICALE 3. THE ENT CONTRACTOR'S		GROU	T AND ADHESIVES		• • • • • • • • • • • • • • • • • • • •
ADHESIVE AND MECHANICAL ANCHORS STRUCTI 1. ADHESIVE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT-RE 500-SD EPOXY" BUILDING (ICC-ES ESR-2322), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRUCTUR STRUCTUR BY SINDEON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNA, OLAR POPROVE DE OULL. ADHESIVE ANCHORS STRULL CONFORM TO STRUCTUR PLESANTON, CALIFORNIA, OR APPROVED EQUAL. ADHESIVE ANCHORS STRULL CONFORM TO INFORM INIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD AND/OR TORQUE TEST LOAD, U.O.N.: INIT ORQUE # 30 R % DIA 3% 2.000 POUNDS 20 FOOT-POUNDS # 40 R % DIA 3% 2.000 POUNDS 20 FOOT-POUNDS # 50 R % DIA 3% 2.000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 6.000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS # 50 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS # 10 NO TESS 51,000 POUNDS 50 FOOT-POUNDS # 10 R % DIA 5% 12,000 POUNDS 50 FOOT-POUNDS <td></td> <td>1.</td> <td>NON—SHRINK GROUT SHALL BE "SIKAGROUT 212", AS MANUFACTURED BY SIKA CORPORATION, LYNDHURST, NEW JERSEY, OR APPROVED EQUAL. NON—SHRINK GROUT SHALL BE NON—METALLIC</td> <td>Certified te With the PF More String</td> <td>ECHN Roje Gen⁻</td>		1.	NON—SHRINK GROUT SHALL BE "SIKAGROUT 212", AS MANUFACTURED BY SIKA CORPORATION, LYNDHURST, NEW JERSEY, OR APPROVED EQUAL. NON—SHRINK GROUT SHALL BE NON—METALLIC	Certified te With the PF More String	ECHN Roje Gen ⁻
1. ADHESNE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT—RE 500—SD EPOXY" IMMEDIAI 1. ADHESNE ANCHORS FOR CONCRETE CONSTRUCTION SHALL USE "HILTI HIT—RE 500—SD EPOXY" IMMEDIAI 1. CCC_ES ESR-25203), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRUCTURE 1. CALIFORNIA, OR APPROVED EQUAL. ADHESIVE ANCHORS SHALL CONFORM TO IMMSONRY 1. COMPANY, INC., PLEXANTION, CALIFORNIA, OR APPROVED EQUAL. ANDESIVE ANCHORS SHALL CONFORM TO IMMSONRY 2. ADHESIVE ANCHORS IN CONCRETE AND MAONSRY SHALL BE INSTALLED WITH THE FOLLOWING COMPARE MINIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD MIN TORQUE 2.5 WOT # 30 R % DIA 3% " 2,000 POUNDS 20 FOOT-POUNDS 2. SHOT # 40 R ½" DIA 5% " 6,000 POUNDS 50 FOOT-POUNDS 1. WEED # 50 R % DIA 5% " 1,000 POUNDS 60 FOOT-POUNDS 1. WEED # 60 R ½" DIA 5% " 1,000 POUNDS 60 FOOT-POUNDS 1. WEED # 60 R ½" DIA 5% " 1,000 POUNDS 50 FOOT-POUNDS 1. WEED # 60 R ½" DIA 9" 15,000 POUNDS 60 FOOT-POUNDS 1. WEED # 70 R ½DI NOT LESS THAN THREE BOLTS, SHALL BE ENSTED VON LESS<		ADHES	SIVE AND MECHANICAL ANCHORS	STRUCTURAL	TES
 2. ADHESIVE ANCHORS IN CONCRETE AND MAONSRY SHALL BE INSTALLED WITH THE FOLLOWING MINIMUM EMBEDMENT AND DIRECT TENSION TEST LOAD AND/OR TORQUE TEST LOAD, U.O.N.: REBAR OR THREADED BOLT MIN EMBED TENSION TEST LOAD MIN TORQUE #3 OR %" DIA 3%" 2,000 POUNDS 20 FOOT-POUNDS #4 OR ½" DIA 4½" 4,000 POUNDS 40 FOOT-POUNDS #5 OR %" DIA 5%" 6,000 POUNDS 50 FOOT-POUNDS #6 OR ½" DIA 6%" 9,000 POUNDS 60 FOOT-POUNDS #7 OR %" DIA 7%" 12,000 POUNDS 60 FOOT-POUNDS #8 OR 1" DIA 9" 15,000 POUNDS FORCENT OF ALL NEW ADHESIVE ANCHORED BOLTS IN EXISTING CONCRETE, BUT NOT LESS THAN TWO BOLTS, SHALL BE SUBJECT TO DIRECT TENSION TEST, AND AN ADDITONAL 20 PERCENT, BUT NOT LESS THAN THREE BOLTS, SHALL BE TESTED USING A TORQUE CALBRATED WRENCH. ANCHORS THAT FAIL THE TEST LOAD SHALL BE REPLACED AND RE-TESTED AT CONTRACTOR'S EXPENSE. 3. ADHESIVE ANCHORS FOR MASONRY CONSTRUCTION SHALL USE "HILTI HIT HY-150 EPOXY" (ICC-ES ESR-1967), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. 4. MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI SPECIFIC STRUCTUR MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. 4. MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI SPECIFIC STRUCTUR MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. 4. MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI SPECIFIC STRUCTUR MANUFACTURED BY SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1717), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR", ICC., FLESANTON, CALIFORNIA, OR APPROVED EQUAL. 		1.	(ICC-ES ESR-2322), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON SET-XP" (ICC-ES ESR-2508), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR APPROVED EQUAL. ADHESIVE ANCHORS SHALL CONFORM TO	IMMEDIATELY STRUCTURAL TE ITEMS MASONRY 1. COMPRESS	BE ESTIN
REBAR OR THREADED BOLT MIN EMBED TENSION TEST LOAD MIN TORQUE 3,000 #3 OR %" DIA 3%" 2,000 POUNDS 20 FOOT-POUNDS 2. SNOT #4 OR ½" DIA 4½" 4,000 POUNDS 40 FOOT-POUNDS 2. SNOT #5 OR %" DIA 5%" 6,000 POUNDS 50 FOOT-POUNDS 7. SNOT #6 OR 3," DIA 6%" 9,000 POUNDS 60 FOOT-POUNDS 7. SNOT #7 OR %" DIA 7%" 12,000 POUNDS 60 FOOT-POUNDS 7. SNOT #8 OR 1" DIA 9" 15,000 POUNDS 80 FOOT-POUNDS 1. TEST 5 PERCENT OF ALL NEW ADHESIVE ANCHORED BOLTS IN EXISTING CONCRETE, BUT NOT LESS 1. TEST 1. TEST THAN TWO BOLTS, SHALL BE SUBJECT TO DIRECT TENSION TEST, AND AN ADDITIONAL 20 2. BASE PERCENT, BUT NOT LESS THAN THREE BOLTS, SHALL BE REPLACED AND RE-TESTED AT 2. BASE 3. ADHESIVE ANCHORS FOR MASONRY CONSTRUCTION SHALL BE REPLACED AND RE-TESTED AT 3. THE EN MINICATE SER-1967), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. 3. THE EN NINCATE SER-1967), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1917), AS 3. THE EN 4. MECHANICAL EXPANSION ANCHORS FOR CONCR		2.		COMPRESS CONCRETE 1. COMPRESS	
 THAN TWO BOLTS, SHALL BE SUBJECT TO DIRECT TENSION TEST, AND AN ADDITIONAL 20 PERCENT, BUT NOT LESS THAN THREE BOLTS, SHALL BE TESTED USING A TORQUE CALIBRATED WRENCH. ANCHORS THAT FAIL THE TEST LOAD SHALL BE REPLACED AND RE-TESTED AT CONTRACTOR'S EXPENSE. 3. ADHESIVE ANCHORS FOR MASONRY CONSTRUCTION SHALL USE "HILTI HIT HY-150 EPOXY" (ICC-ES ESR-1967), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. 4. MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI KWIK BOLT TZ WEDGE ANCHOR" (ICC-ES ESR-1917), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1771), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR APPROVED EQUAL. 			#3 OR ¾" DIA 3¾" 2,000 POUNDS 20 FOOT-POUNDS #4 OR ½" DIA 4½" 4,000 POUNDS 40 FOOT-POUNDS #5 OR ⅛" DIA 5½" 6,000 POUNDS 50 FOOT-POUNDS #6 OR ¾" DIA 6¾" 9,000 POUNDS 60 FOOT-POUNDS #7 OR ⅛" DIA 7⅛" 12,000 POUNDS 60 FOOT-POUNDS	SPECIFIED 3,000 PSI 2. SHOTCRETE REINFORCING A 1. WELDABILIT CONFORMS STRUCTURAL ST 1. TESTING C	OR TES ND F Y OF WIT
 ESR-1967), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR APPROVED EQUAL. MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI SPECIFIC KWIK BOLT TZ WEDGE ANCHOR" (ICC-ES ESR-1917), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1771), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR APPROVED EQUAL. STRUCTURA APPROVED EQUAL. 			THAN TWO BOLTS, SHALL BE SUBJECT TO DIRECT TENSION TEST, AND AN ADDITIONAL 20 PERCENT, BUT NOT LESS THAN THREE BOLTS, SHALL BE TESTED USING A TORQUE CALIBRATED WRENCH. ANCHORS THAT FAIL THE TEST LOAD SHALL BE REPLACED AND RE-TESTED AT	2. BASE META	L TH
 MECHANICAL EXPANSION ANCHORS FOR CONCRETE AND MASONRY CONSTRUCTION SHALL BE "HILTI SPECIFIC STRUCTUR KWIK BOLT TZ WEDGE ANCHOR" (ICC-ES ESR-1917), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1771), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR STRUCTUR APPROVED EQUAL. 		3.		3. THE ENGINE	
2. MAT F		4.	KWIK BOLT TZ WEDGE ANCHOR" (ICC-ES ESR-1917), AS MANUFACTURED BY HILTI INC., TULSA, OKLAHOMA, OR "SIMPSON STRONG-BOLT WEDGE ANCHOR" (ICC-ES ESR-1771), AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC., PLEASANTON, CALIFORNIA, OR	SPECIFICATION STRUCTURAL RESPONSIBILI STRUCTURAL OE FOUNDATIONS	NS / SYS TY F
	SP	ECIAL INS	PECTION, TESTING, STRUCTURAL OBSERVATION, AND SUBMITTALS	1.ISOLATED &2.MAT FOUND3.PIERS, CAIS4.RETAINING)ATIO SSON

WHERE INDICATED WITH AN "X", THE FOLLOWING ITEMS SHALL BE INSPECTED IN ACCORDANCE 1. WITH SFBC 1704 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED SPECIAL INSPECTION AGENCY. "C" INDICATES CONTINUOUS SPECIAL INSPECTION AND "P" INDICATES PERIODIC SPECIAL INSPECTION. THE SPECIAL INSPECTION AGENCY SHALL SEND COPIES OF ALL SPECIAL INSPECTION REPORTS DIRECTLY TO THE RESIDENT ENGINEER, ARCHITECT, ENGINEER, AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET THE PROJECT SPECIFICATIONS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ENGINEER.

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	FICATION AND INSPECTION	C	P	NOTES
CON	CRETE CONSTRUCTION			
1.	INSPECTION OF REINFORCING STEEL PLACEMENT		Х	
2.	INSPECTION OF REINFORCING STEEL WELDING			
	2.1. VERIFICATION OF WELDABILITY			
	2.2. REINFORCING STEEL RESISTING FLEXURAL &			
	AXIAL FORCES IN INTERMEDIATE AND SPECIAL			
	MOMENT FRAMES, AND BOUNDARY ELEMENTS OF			
	SPECIAL REINFORCED CONCRETE SHEAR WALLS			
	2.3. SHEAR REINFORCEMENT			
	2.4. OTHER REINFORCING STEEL			
3.	INSPECT BOLTS TO BE INSTALLED IN CONCRETE	X		
	PRIOR TO AND DURING PLACEMENT OF CONCRETE			
4.	VERIFY USE OF REQUIRED DESIGN MIX		Х	
5.	FABRICATE SPECIMENS FOR STRENGTH TESTS,		Х	
	PERFORM SLUMP AND AIR CONTENT TESTS, AND			
	DETERMINE TEMPERATURE OF CONCRETE			
6.	INSPECTION OF CONCRETE & SHOTCRETE PLACEMENT	Х		
7.	INSPECTION OF CONCRETE CURING		χ	
8.	INSPECTION OF PRESTRESSED CONCRETE			
	8.1. APPLICATION OF PRESTRESSING FORCES			
	8.2. GROUTING OF BONDED PRESTRESSING TENDONS			
9.	ERECTION OF PRECAST CONCRETE MEMBERS			
10.				
				PRIOR TO REMOVAL OF FORMS
11.	INSPECT FORMWORK FOR SHAPE, LOCATION, AND		Х	
	DIMENSIONS OF THE CONCRETE MEMBER BEING			
	FORMED			
L				

VER	IFICATION AND INSPECTION	C	P	NOTES
SOIL	S			
1.	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIRED BEARING CAPACITY		X	BY GEOTECHNICAL ENGINEER
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND REACHED PROPER MATERIAL		X	BY GEOTECHNICAL ENGINEER
3.	PERFORM CLASSIFICATION AND TESTING OF ENGINEERED FILL MATERIAL		X	BY GEOTECHNICAL ENGINEER
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF ENGINEERED FILL		X	BY GEOTECHNICAL ENGINEER
5.	PRIOR TO PLACEMENT OF ENGINEERED FILL, OBSERVE SUBGRADE & VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		X	BY GEOTECHNICAL ENGINEER

OTHER



WHERE INDICATED WITH AN "X", THE FOLLOWING ITEMS SHALL BE SAMPLED AND/OR TESTED BY A CERTIFIED TECHNICIAN FROM AN ESTABLISHED MATERIALS TESTING LABORATORY IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, GENERAL NOTES, OR PREVAILING BUILDING, WHICHEVER IS MORE STRINGENT. ALL MATERIAL SAMPLING AND TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM REQUIREMENTS. THE MATERIALS TESTING LABORATORY SHALL SEND COPIES OF ALL STRUCTURAL TESTING REPORTS DIRECTLY TO THE RESIDENT ENGINEER, ARCHITECT, ENGINEER, AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET THE PROJECT SPECIFICATION SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ENGINEER.

TRUCTURAL TESTING FOR SEISMIC RESISTANCE REQ'D NOTES COMPRESSIVE STRENGTH TESTS FOR MINIMUM COMPRESSIVE STRENGTH, fM' AND fAAC' COMPRESSIVE STRENGTH TESTS FOR CONCRETE WITH Х SPECIFIED MINIMUM COMPRESSIVE STRENGTH, fc', OF 3,000 PSI OR GREATER AT 28 DAYS . SHOTCRETE TEST PANELS AND CORE SAMPLES REINFORCING AND PRESSTRESSING STEEL WELDABILITY OF REINFORCEMENT, EXCEPT THAT WHICH CONFORMS WITH ASTM A706 TRUCTURAL STEEL TESTING CONTAINED IN THE QUALITY ASSURANCE PLAN THIS INCLUDES NON-DESTRUCTIVE TESTING (NDT) OF WELDS . BASE METAL THICKER THAN 11/2" ULTRASONIC TESTING FOR DISCONTINUITIES BEHIND & ADJACENT TO WELDS SUBJECT TO THROUGH-THICKNESS WELD SHRINKAGE STRAINS

THE ENGINEER OF RECORD SHALL PROVIDE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, INDICATED WITH AN "X" BELOW, FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS AT SIGNIFICANT CONSTRUCTION STAGES AND AT THE COMPLETION OF THE STRUCTURAL SYSTEM. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS AND SPECIAL INSPECTIONS REQUIRED BY THE SFBC.

STRUCTURAL OBSERVATION REQUIREMENTS	REQ'D	NOTES
FOUNDATIONS		*******
1. ISOLATED & CONTINUOUS FOOTINGS, STEM WALLS	X	
2. MAT FOUNDATIONS	X	
3. PIERS, CAISSONS, PILES, PILE CAPS		
4. RETAINING WALLS, HILLSIDE CONSTRUCTION	т. — Халасана	
	and the second second	· · · · · ·
SHEAR WALLS		
1. LIGHT-FRAMED SHEAR WALLS, INCLUDING HOLDOWN		
INSTALLATION AND SHEATHING NAILING		
2. CONCRETE SHEAR WALLS, INCLUDING REINFORCING	X	
STEEL PLACEMENT AND CONCRETE PLACEMENT		
3. MASONRY SHEAR WALLS, INCLUDING REINFORCING		
STEEL PLACEMENT AND GROUT PLACEMENT		· · · · · · · · · · · · · · · · · · ·
4. STEEL SHEAR WALLS		
MOMENT-RESISTING FRAMES		
1. CONCRETE MOMENT-RESISTING FRAMES, INCLUDING		
REINFORCING STEEL PLACEMENT & CONCRETE PLACEMENT		
2. STEEL MOMENT-RESISTING FRAMES		
BRACED FRAMES		
1. STEEL BRACED FRAMES		
·		
HORIZONTAL ROOF AND FLOOR DIAPHRAGMS		·
1. CONCRETE	X	
2. STEEL DECK, CONCRETE ON STEEL DECK		·
3. WOOD	_	· · · · ·
4. CHORDS AND/OR COLLECTORS		
		· · · · · · · · · · · · · · · · · · ·
OTHER		

4. WHERE INDICATED WITH AN "X" BELOW, THE CONTRACTOR SHALL SUBMIT CERTIFICATES OF CONFORMANCE, SHOP DRAWINGS, CALCULATIONS, AND DETAILS TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. WHERE CALCULATIONS AND DETAILS ARE REQUIRED, THE SUBMITTAL SHALL BE SEALED AND SIGNED BY A REGISTERED DESIGN PROFESSIONAL IN THE STATE OF CALIFORNIA. FOR ADDITIONAL INFORMATION REGARDING SUBMITTALS, SEE SPECIFICATIONS.

ПЕМ	CERTIFICATES	shop Drawings	CALCULATIONS & DETAILS	REMARKS
CONCRETE, REINFORCING	X	Х		
CONCRETE, MIX DESIGN		Х		
CONCRETE, CEMENT	X			
CONCRETE, FINE AGGREGATES	X			
CONCRETE, COARSE AGGREGATES	X			
CONCRETE, ADMIXTURES	Х			
SHOTCRETE, MIX DESIGN				
PRECAST CONCRETE MEMBERS				
MASONRY, REINFORCING				
MASONRY, MORTAR MIX DESIGN		1.14		
MASONRY, GROUT MIX DESIGN		na serie de la companya d companya de la companya d		
MASONRY, UNITS				
MASONRY, LIME	-		-	
STRUCTURAL STEEL				
OPEN WEB JOISTS		÷ 2.		
METAL DECKING WITH STUD LAYOUT				
COLD-FORMED STRUCTURAL STEEL				
METAL STAIRS	× ·			
TEMPORARY SHORING SYSTEM		Х	Х	

A.B. ACI ADDT'L ADJ AISC ARCH ASTM AWS & @	ADJACENT AMERICAN INSTITUTE OF STEEL CONSTRUCTION
BAL BLDG BLKG BM B, BOT B.O. B.O.E. B.O.F. B.O.S. B.O.W. B.S. BET, BTWN	BOTTOM OF BOTTOM OF EXCAVATION BOTTOM OF FOOTING BOTTOM OF STEEL BOTTOM OF WALL BOTH SIDE
CBC	CALIFORNIA BUILDING CODE
CCSF	CITY AND COUNTY OF SAN FRANCISCO
CHK PL	CHECKERED PLATE
C.J.	CONTROL JOINT
C.J.P.	COMPLETE JOINT PENETRATION
€	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNITS
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CTR	CENTER
DBL	DOUBLE
DET	DETAIL
DIA, Ø	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
DN	DOWN
DO	DITTO
DWG	DRAWING
(E)	EXISTING
EA	EACH
E.K.	EACH FACE
E.J.	EXPANSION JOINT
EL, ELEV	ELEVATION
ELEC	ELECTRICAL
EMBED	EMBEDMENT
E.N.	EDGE NAILING
EQ	EQUAL
EQUIP	EQUIPMENT
E.S.	EACH SIDE
E.W.	EACH WAY
EXT	EXTERIOR
F.F.	FINISH FLOOR
F.G.	FINISH GRADE
FIN	FINISH
FL	FLOOR
FNDN	FOUNDATION
F.O.C.	FACE OF CONCRETE
F.O.F.	FACE OF FINISH
F.O.S.	FACE OF STUD
F.O.W.	FACE OF WALL
FRP	FIBER REINFORCED PLASTIC
FT	FOOT, FEET
FT	FOOTING
GA	GAGE
GALV	GALVANIZED
H, HORIZ	HORIZONTAL
HDPE	HIGH DENSITY POLYETHYLENE
H.R.	HAND RAIL
H.S.B.	HIGH STRENGTH BOLT
HSS	HOLLOW STRUCTURAL SECTION
icc	INTERNATIONAL CODE COUNCIL
I.d.	INSIDE DIAMETER
I.f.	INNER FACE
In	INCHES
Info	INFORMATION
Insul	INSULATION
Int	INTERIOR

ABBREVIATIONS

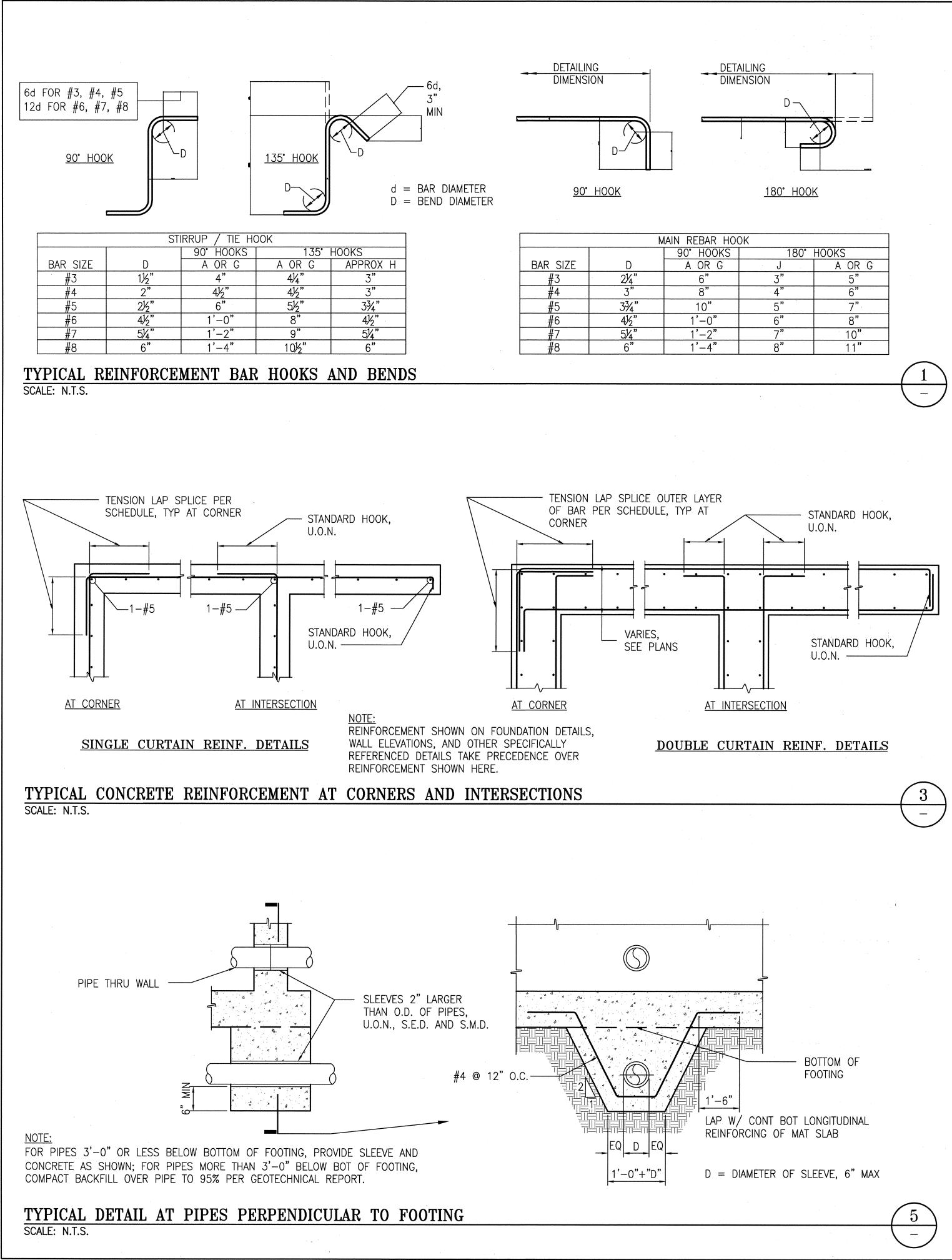
_		DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer
JT L	JOINT ANGLE	BUILDING DESIGN &
L.L.H. L.L.V. L.O.L.	LONG LEG HORIZONTAL LONG LEG VERTICAL LAYOUT LINE	CONSTRUCTION
LVL L.W.C.	LAMINATED VENEER LUMBER LIGHT WEIGHT CONCRETE	A AND COUNTA ON
MAT'L MAX MECH	MATERIAL MAXIMUM MECHANICAL	E NAS
MET	MECHANICAL METAL MILLIMETER	E A
MIN MISC	MINIMUM MISCELLANEOUS	A PAS - 0351
(N) N.D.T.	NEW NON-DESTRUCTIVE TESTING	Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager 30 Van Ness Avenue Suite 4100
N.I.C. N.T.S.	NOT IN CONTRACT NOT TO SCALE	San Francisco, CA (415) 557-4700 94102-6028 Fax (415) 5574701
NO. NOM N.W.C.	NUMBER NOMINAL	Project
0.C.	NORMAL WEIGHT CONCRETE	
0.D. 0.F.	OUTSIDE DIAMETER OUTER FACE	2008 PARK BOND RESTROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN
0.H. OPNG	OPPOSITE HAND OPENING	DUPONT TENNIS COURTS RESTROOMS
OPP	OPPOSITE	336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007
P.J.P. PL P.L.	PARTIAL JOINT PENETRATION PLATE PROPERTY LINE	
PLF PLYWD	PROPERTY LINE POUNDS PER LINEAR FOOT PLYWOOD	
PSI PSF	POUNDS PER SQUARE INCH POUNDS PER SQUARE FOOT	
PSL	PARALLEL STRAND LUMBER	Consultant
R R.C.	RADIUS RELATIVE COMPACTION	
RDWD REINF	REDWOOD REINFORCING, REINFORCEMENT, REINFORCED	INFRASTRUCTURE DIVISION DEPARTMENT OF PUBLIC WORKS CITY & COUNTY OF SAN FRANCISCO
REQ'D RET R.O.	REQUIRED RETAIN, RETAINING ROUGH OPENING	30 VAN NESS AVENUE, 5TH FLOOR SAN FRANCISCO, CA 94102-6028
S.A.D.	SEE ARCHITECTURAL DRAWING	DESIGNED BY: SL SL DATE 09/2012
S.E.D. S.M.D.	SEE ELECTRICAL DRAWING SEE MECHANICAL DRAWING	DRAWN BY: BH 09/2012
S.P.D. Sched	SEE PLUMBING DRAWING SCHEDULE	CHECKED BY: JC JC 09/2012
sec SFBC	SECTION SAN FRANCISCO BUILDING CODE	SECTION MANAGER IO/ DATE:
SHT SHTG	SHEET SHEATHING	DEPUTY DIVISION MANAGER: DATE:
sim S.P.G. Spec	SIMILAR SLAB-ON-GRADE SPECIFICATION	DIVISION MANAGER: DATE:
S.S. SQ	STAINLESS STEEL SQUARE	BID & CONSTRUCTION
STD STIFF	STANDARD STIFFENER	No. Date Revisions
stir Stl	STIRRUP STEEL	
STRUCT SYM	STRUCTURAL SYMMETRICAL	
Т Т&В	TOP	
THK THRU	TOP AND BOTTOM THICK THROUGH	Section Head
T.O. T.O.C.	TOP OF TOP OF CONCRETE	Proj. Mgr. <u>M. YEE</u>
T.O.S. T.O.W.	TOP OF STEEL TOP OF WALL	Proj. Arch. T. LEUNG / No. S 409
TYP U.O.N.	TYPICAL UNLESS OTHERWISE NOTED	Drawn
V, VERT	VERTICAL	SEPTEMBER 2012 Phase PERMIT SET
V.I.F.	VERIFY IN FIELD	
W/ W/O WD	WITH WITHOUT WOOD	Drawing Title
WF W.R.T.	WIDE FLANGE WITH RESPECT TO	STRUCTURAL
WT W.W.F.	WEIGHT WELDED WIRE FABRIC	GENERAL NOTES
		S1.3
1		

NONE

3092V-2

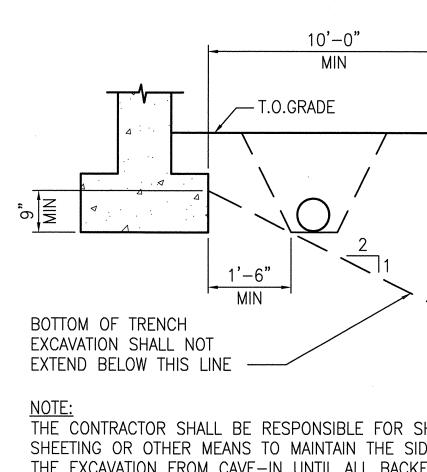
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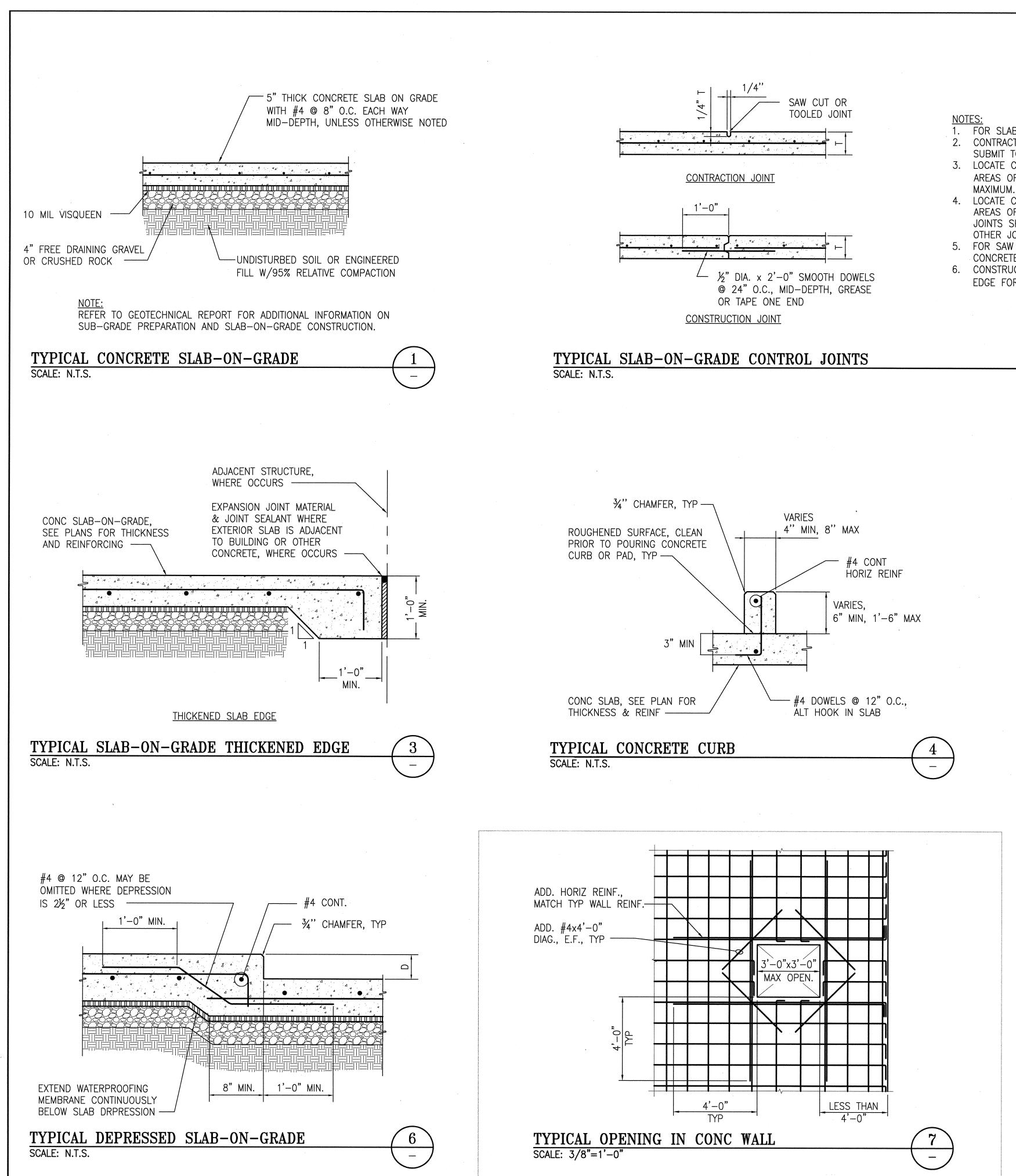
Job No.



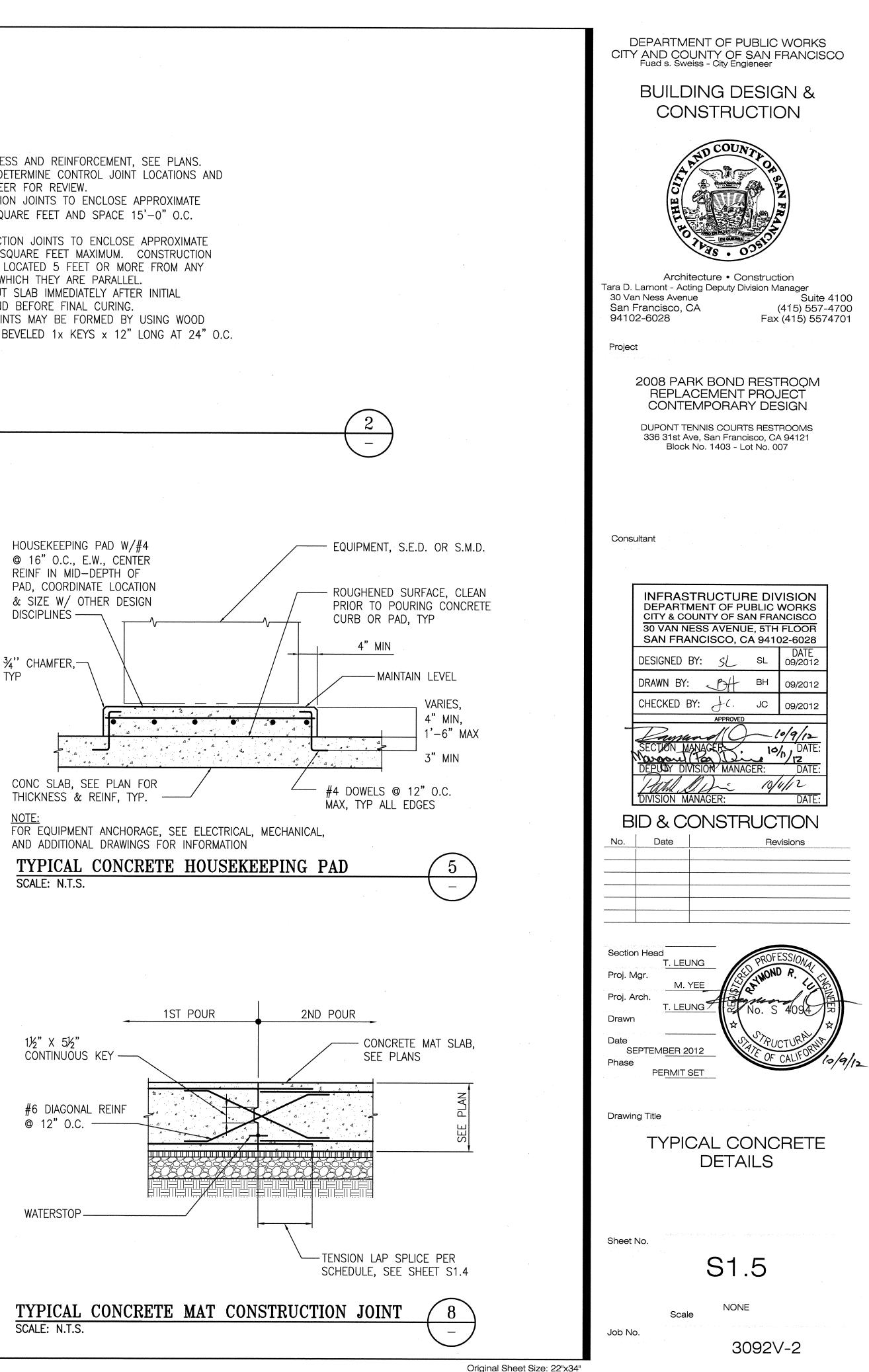
$\frac{f'c = 3,000 \text{ PSI}}{M\text{IN DEVELOPMENT LENGTH} M\text{IN LAP SPLICE LENGTH}} \\ \hline \frac{M\text{IN DEVELOPMENT LENGTH} HOOKED}{STRAIGHT} HOOKED TOP OTHER#3 1'-10" 1'-5" 0'-9" 2'-4" 1'-10"#4 2'-5" 1'-10" 0'-11" 3'-1" 2'-5"#5 3'-0" 2'-4" 1'-2" 3'-11" 3'-0"#6 3'-7" 2'-9" 1'-5" 4'-8" 3'-7"#7 5'-3" 4'-0" 1'-8" 6'-9" 5'-3"#8 6'-0" 4'-7" 1'-10" 7'-9" 6'-0"1. ALL REINFORCING BARS SHALL BE DEVELOPED OR LAP SPLICED AS SHOWN, U.O.N.2. LAP SPLICE LOCATIONS SHALL BE STAGERED WHENEVER POSSIBLE.3. TOP BARS ARE HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12 INCHES OFFRESH CONCRETE IS CAST BELOW THE LAP SPLICE SCHEDULE 2SCALE: N.T.S.$	<section-header><text><section-header><image/><text><text><text><text><text></text></text></text></text></text></section-header></text></section-header>
$\frac{1'c = 4,000 \text{ PSI}}{\text{MIN LAP SPLICE LENGTH}}$ $\frac{1'r = 4,000 \text{ PSI}}{\text{MIN LAP SPLICE LENGTH}}$ $1'r = 1'r = 1'$	Consultant INFRASTRUCTURE DIVISION DEPARTMENT OF PUBLIC WORKS CITY & COUNTY OF SAN FRANCISCO 30 VAN NESS AVENUE, 5TH FLOOR SAN FRANCISCO, CA 94102-6028 DESIGNED BY: SL SL DESIGNED BY: SL SU DESIGNED BY: SL JO DESIGNED BY: SL JO DESIGNED BY: SL JO DESIGNED BY: SL JO DEPROVED IO JO DEPUNY DIVISION MANAGER: DATE: DEPUNY DIVISION MANAGER: DATE: DIVISION MANAGER: DATE:
	<section-header> Section Head I. LEUNG Proj. Mgr. M. YEE Proj. Arch. I. LEUNG Drawn I. LEUNG Date SEPTEMBER 2012 Prase PERMIT SET Drawing Title Drawn Drawing Title Sheet No. State No. No. S State No.</section-header>

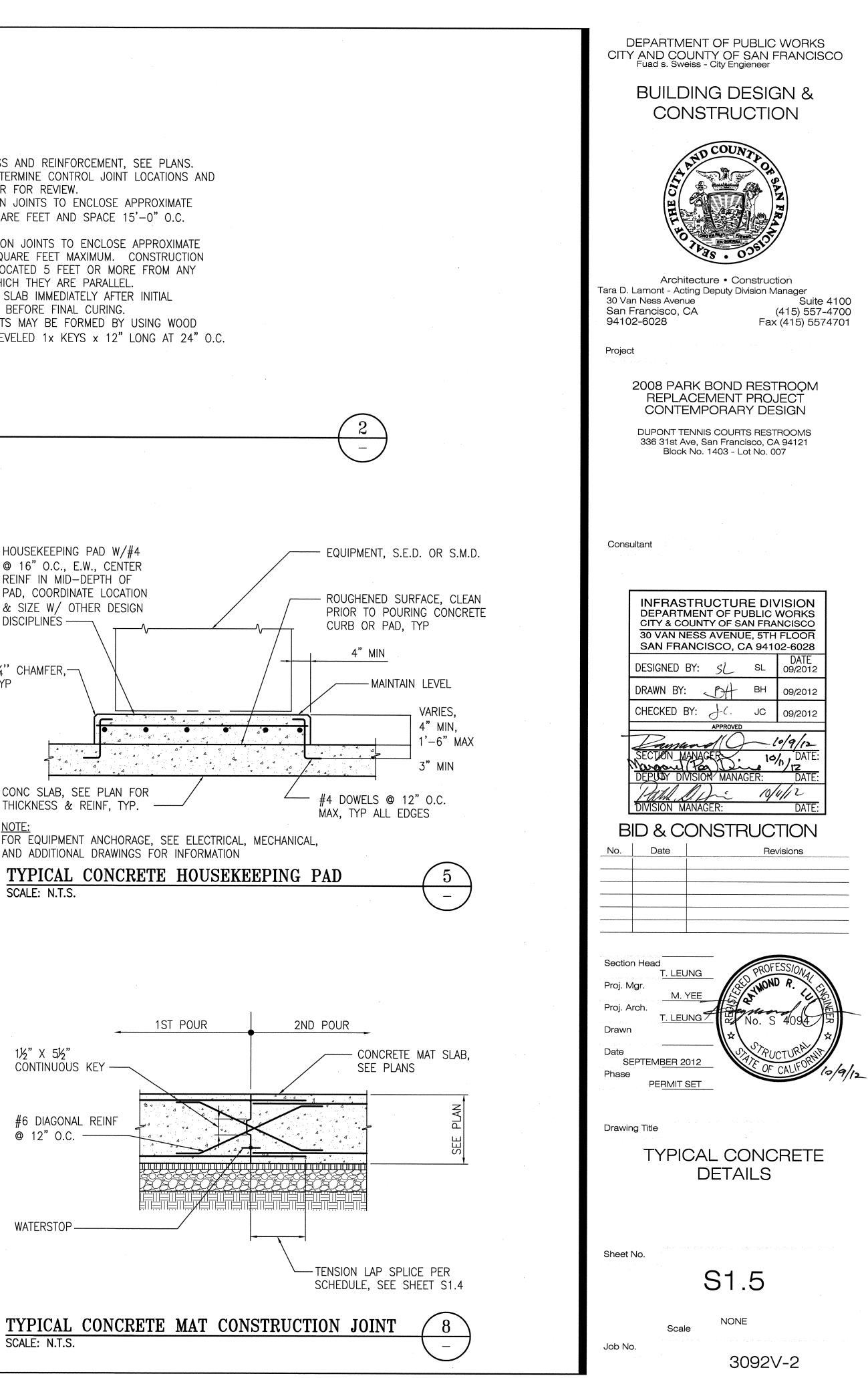
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	<text><section-header><section-header><section-header><section-header><text><text><text><text></text></text></text></text></section-header></section-header></section-header></section-header></text>
DEVELOPMENT LENGTH & LAP SPLICE SCHEDULE 2	CONTEMPORARY DESIGN DUPONT TENNIS COURTS RESTROOMS 336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007
$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Consultant
#5 $2'-7"''$ $2'-0"'$ $1'-0"''$ $3'-5"''$ $2'-7"'''$ #6 $3'-1"''$ $2'-5"''$ $1'-3"''$ $4'-1"''''$ $3'-1"'''''''''''''''''''''''''''''''''''$	30 VAN NESS AVENUE, 5TH FLOOR SAN FRANCISCO, CA 94102-6028 DESIGNED BY: SL DATE DESIGNED BY: SL SL DRAWN BY: BH 09/2012 CHECKED BY: J.C. JC APPROVED
FRESH CONCRETE IS CAST BELOW THE LAP SPLICE.	SECTION MANAGER: 10/11/12 DEPUTY DIVISION MANAGER: DATE: JULIA 10/11/12 DIVISION MANAGER: DATE: BID & CONSTRUCTION No. Date Revisions
	Section Head
10'-0" TRENCH MIN EXCAVATION	T. LEUNG PROILESSION Proj. Mgr. M. YEE Proj. Arch. T. LEUNG Drawn T. LEUNG Date SEPTEMBER 2012 Phase PERMIT SET Drawing Title Drawing Title
BOTTOM OF TRENCH EXCAVATION SHALL NOT EXTEND BELOW THIS LINE	TYPICAL CONCRETE DETAILS
NOTE: THE CONTRACTOR SHALL BE RESPONSIBLE FOR SHORING, SHEETING OR OTHER MEANS TO MAINTAIN THE SIDES OF THE EXCAVATION FROM CAVE-IN UNTIL ALL BACKFILL IS COMPLETED PER SPECIFICATIONS.	Sheet No. S1.4
TYPICAL DETAIL AT PIPES PARALLEL TO FOOTING 6 SCALE: N.T.S.	NONE Scale Job No.
Original Sheet Size: 22"x34"	3092V-2

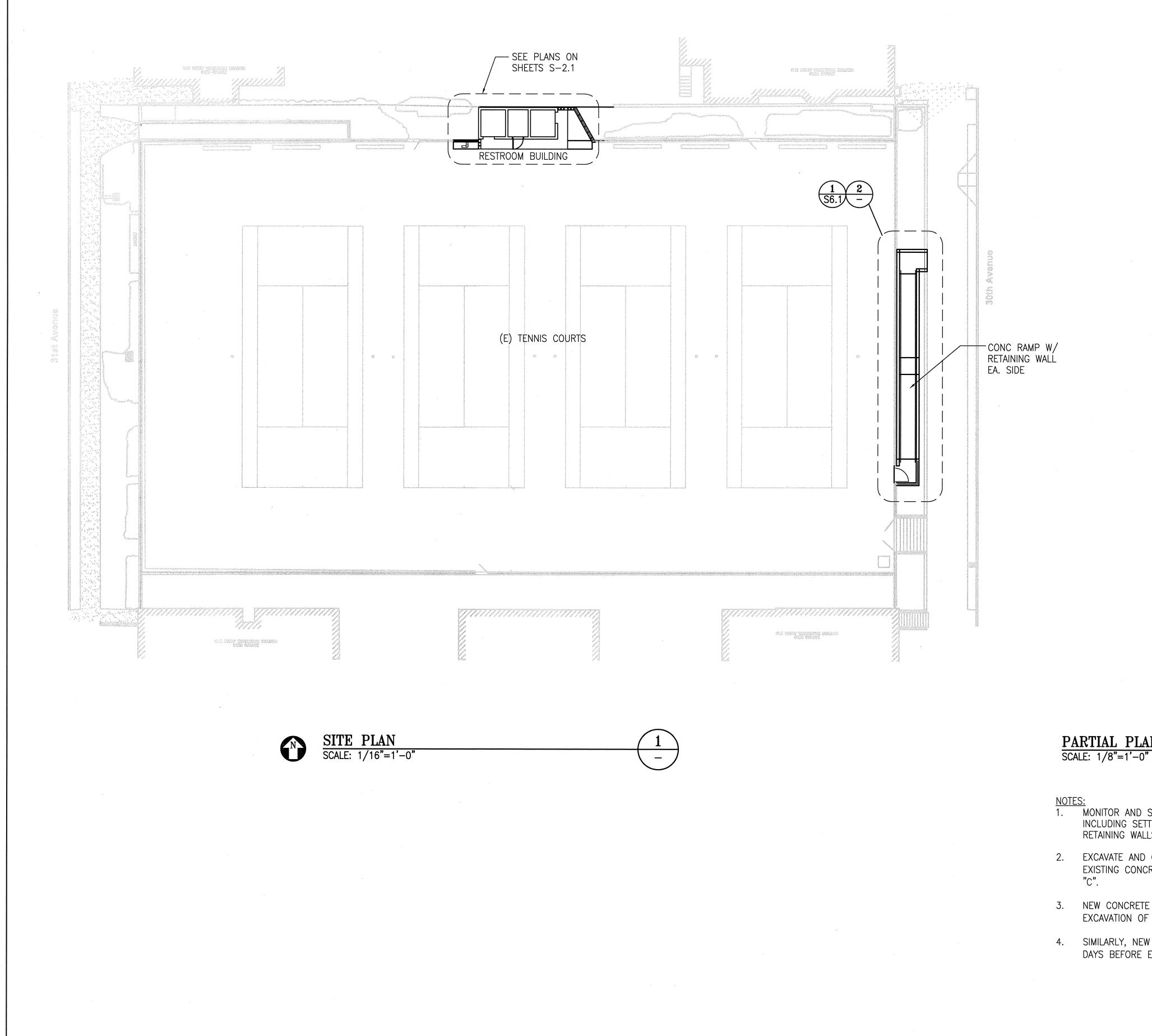


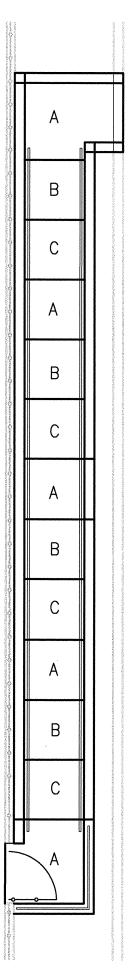


- FOR SLAB THICKNESS AND REINFORCEMENT, SEE PLANS. 2. CONTRACTOR TO DETERMINE CONTROL JOINT LOCATIONS AND
- SUBMIT TO ENGINEER FOR REVIEW. 3. LOCATE CONTRACTION JOINTS TO ENCLOSE APPROXIMATE AREAS OF 600 SQUARE FEET AND SPACE 15'-0" O.C.
- 4. LOCATE CONSTRUCTION JOINTS TO ENCLOSE APPROXIMATE AREAS OF 3.600 SQUARE FEET MAXIMUM. CONSTRUCTION JOINTS SHALL BE LOCATED 5 FEET OR MORE FROM ANY OTHER JOINT TO WHICH THEY ARE PARALLEL.
- 5. FOR SAW CUT, CUT SLAB IMMEDIATELY AFTER INITIAL CONCRETE SET AND BEFORE FINAL CURING.
- 6. CONSTRUCTION JOINTS MAY BE FORMED BY USING WOOD EDGE FORM WITH BEVELED 1x KEYS x 12" LONG AT 24" O.C.



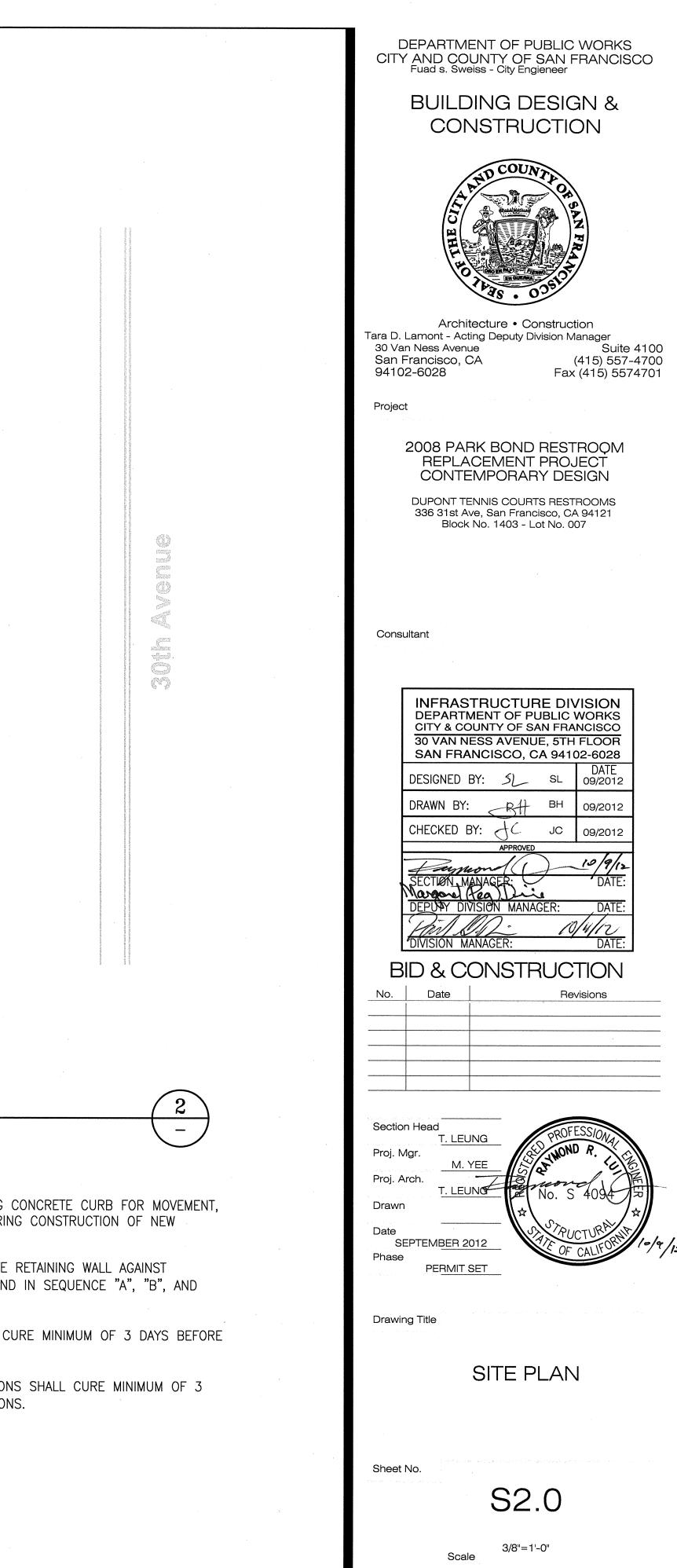




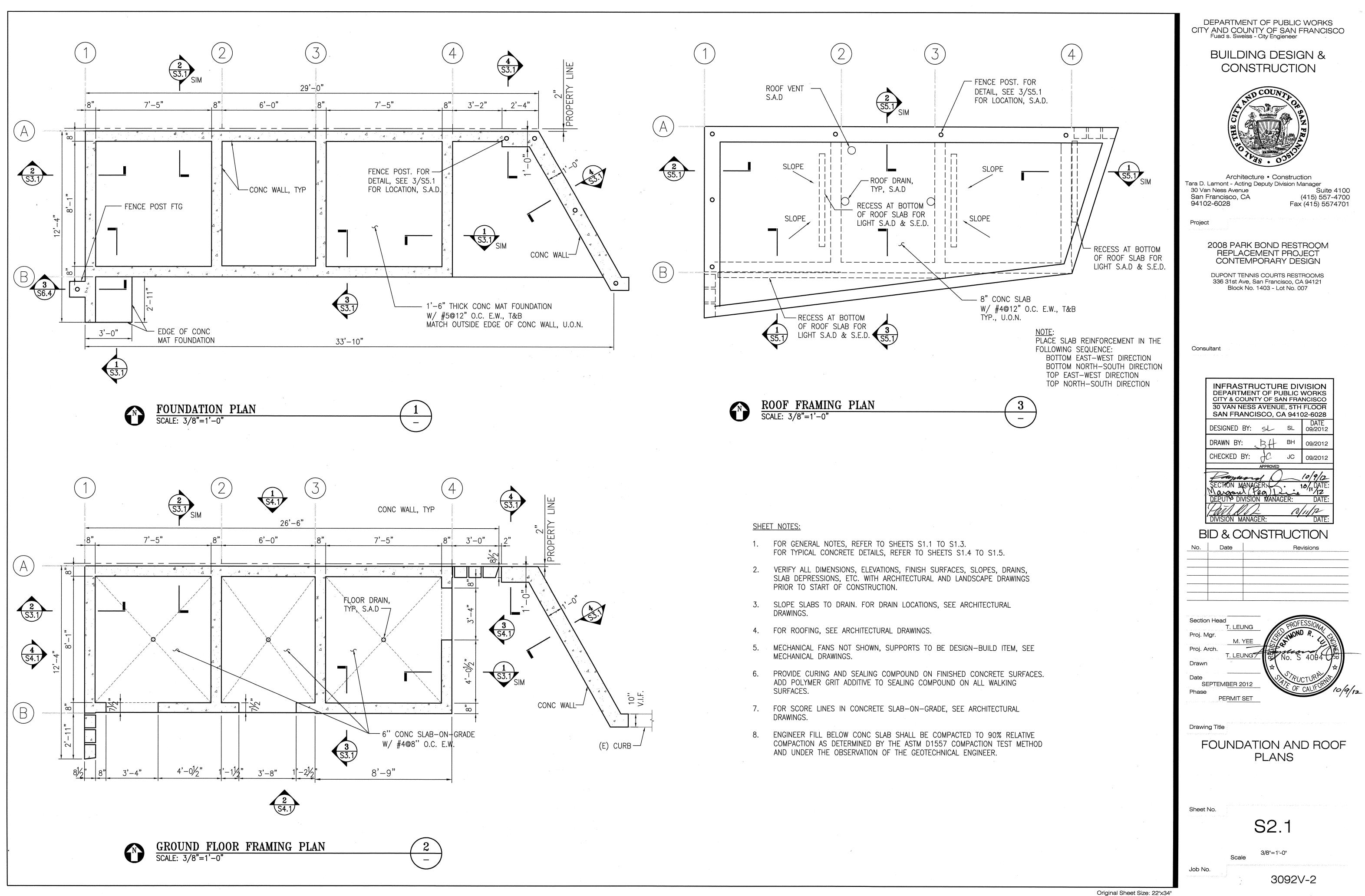


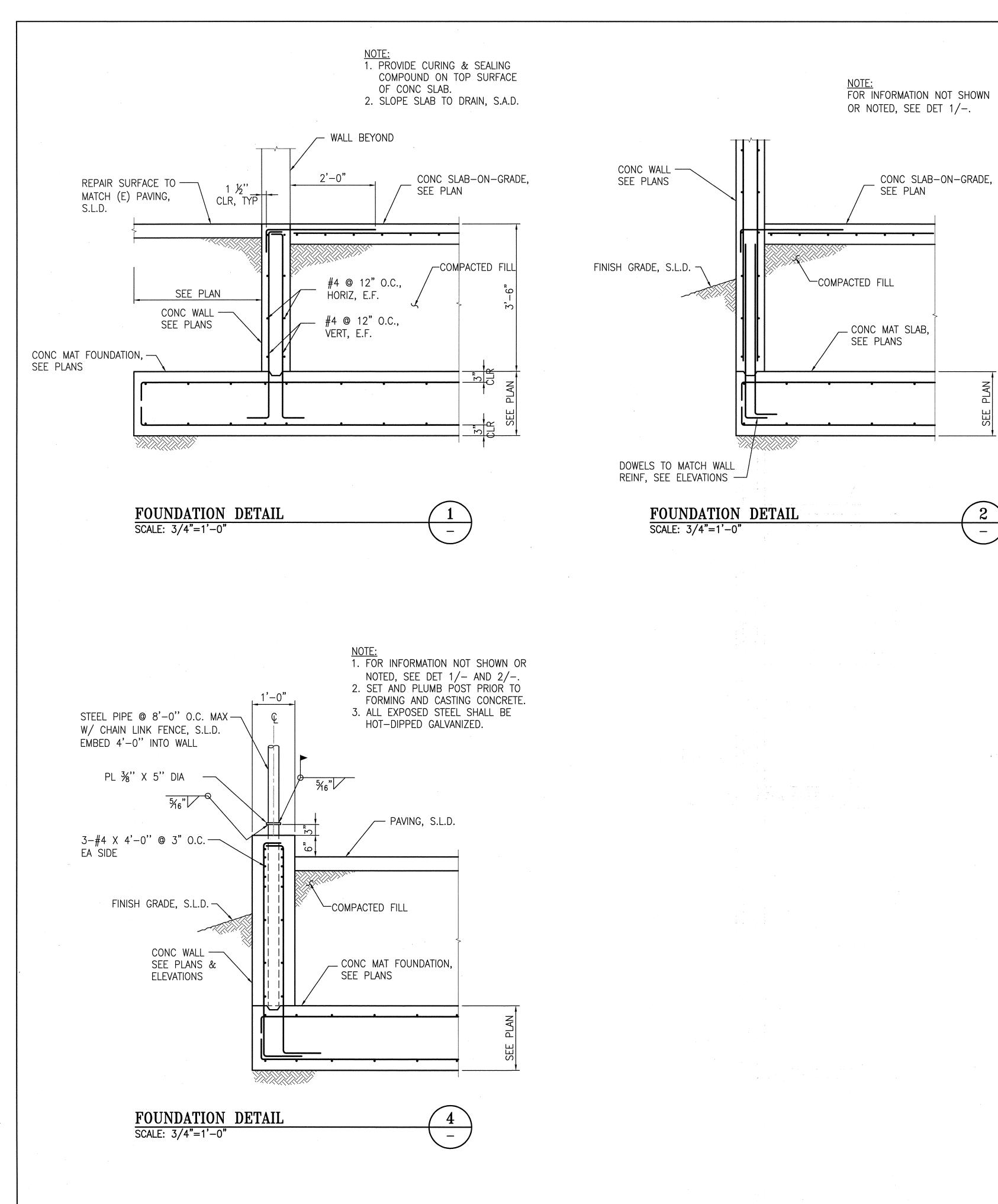
PARTIAL PLAN

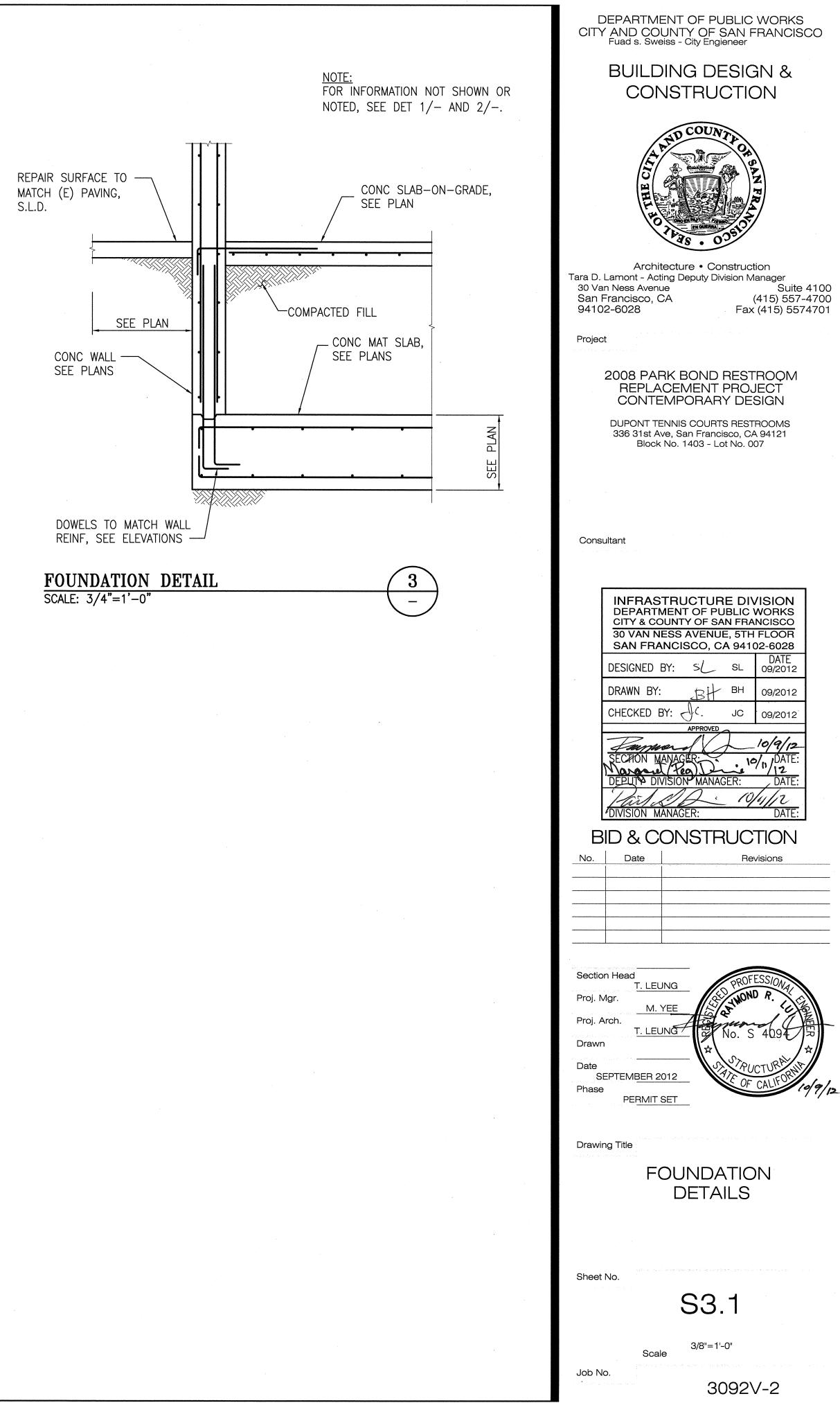
- 1. MONITOR AND SURVEY EXISTING ADJOINING CONCRETE CURB FOR MOVEMENT, INCLUDING SETTLEMENT, BEFORE AND DURING CONSTRUCTION OF NEW RETAINING WALLS.
- 2. EXCAVATE AND CONSTRUCT NEW CONCRETE RETAINING WALL AGAINST EXISTING CONCRETE CURB IN SECTIONS AND IN SEQUENCE "A", "B", AND
- 3. NEW CONCRETE IN "A" LOCATIONS SHALL CURE MINIMUM OF 3 DAYS BEFORE EXCAVATION OF "B" LOCATIONS.
- 4. SIMILARLY, NEW CONCRETE IN "B" LOCATIONS SHALL CURE MINIMUM OF 3 DAYS BEFORE EXCAVATION OF "C" LOCATIONS.



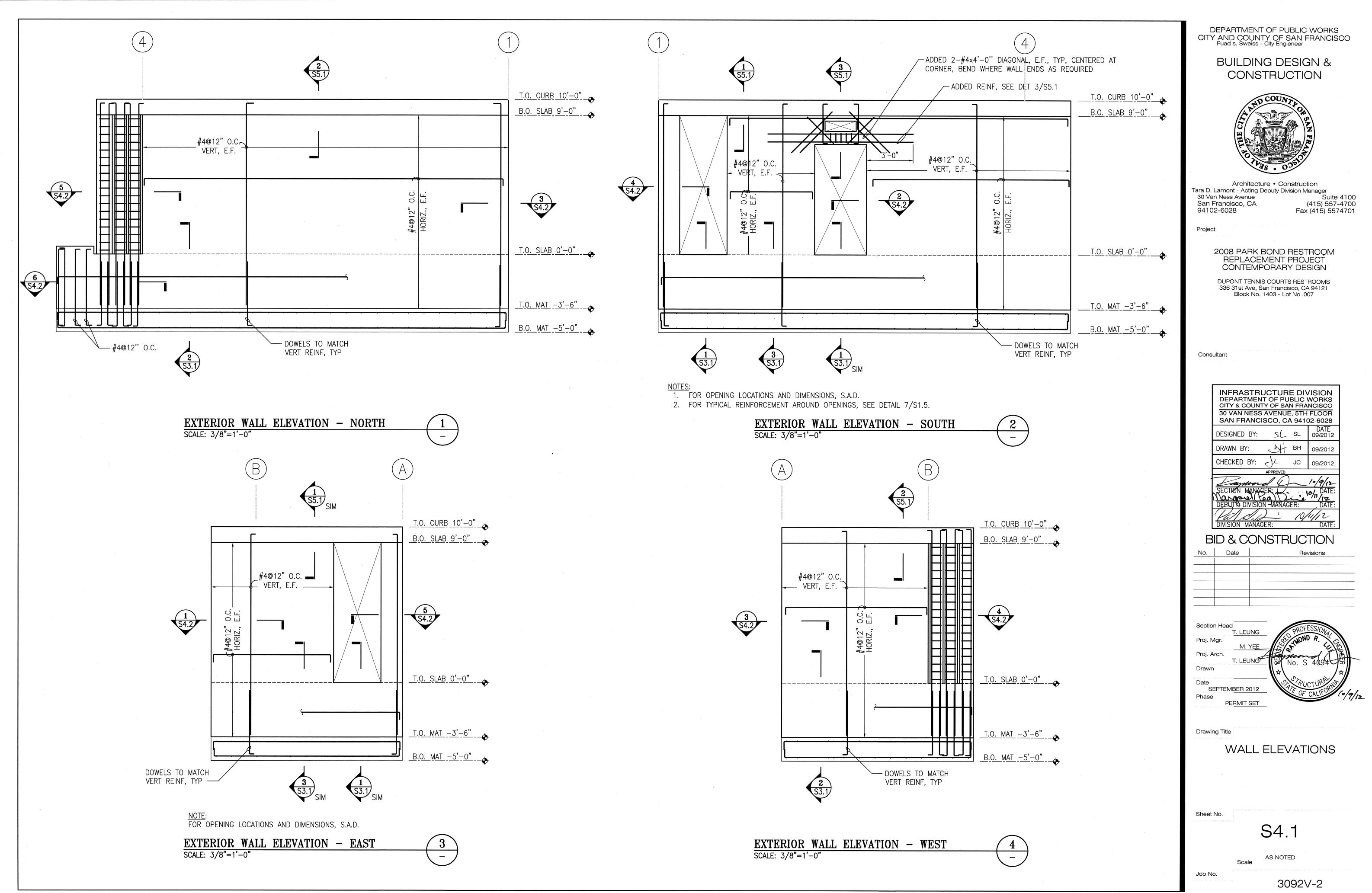
Job No.

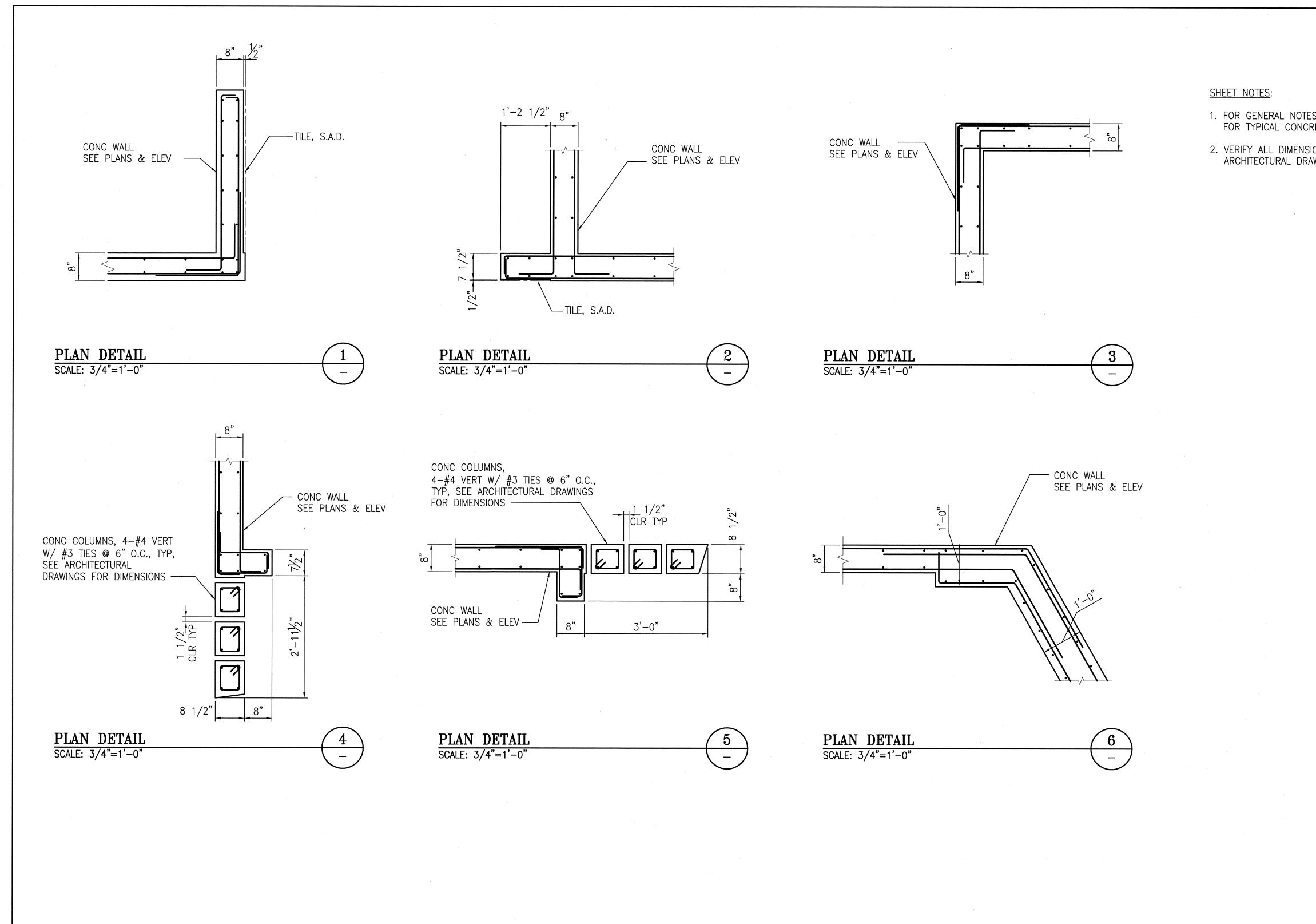






Original Sheet Size: 22"x34"





DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer

FOR GENERAL NOTES, REFER TO SHEETS S1.1 TO S1.3. FOR TYPICAL CONCRETE DETAILS, REFER TO SHEETS S1.4 TO S1.5. VERIFY ALL DIMENSIONS, ELEVATIONS, FINISH SURFACES, ETC. WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION.

BUILDING DESIGN & CONSTRUCTION



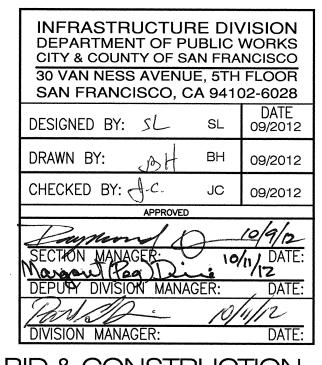
Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager 30 Van Ness Avenue Suite 4100 San Francisco, CA (415) 557-4700 94102-6028 Fax (415) 5574701

Project

2008 PARK BOND RESTROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN

DUPONT TENNIS COURTS RESTROOMS 336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007

Consultant



BID & CONSTRUCTION

No.	Date	Revisions
[

Section Head T. LEUNG	PROFESSIONAL
Proj. Mgr. M. YEE	STHOND P. LE
Proj. Arch. T. LEUNG	No S 4094
Drawn	*
Date	22 PUCTURA NE
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Drawing Title

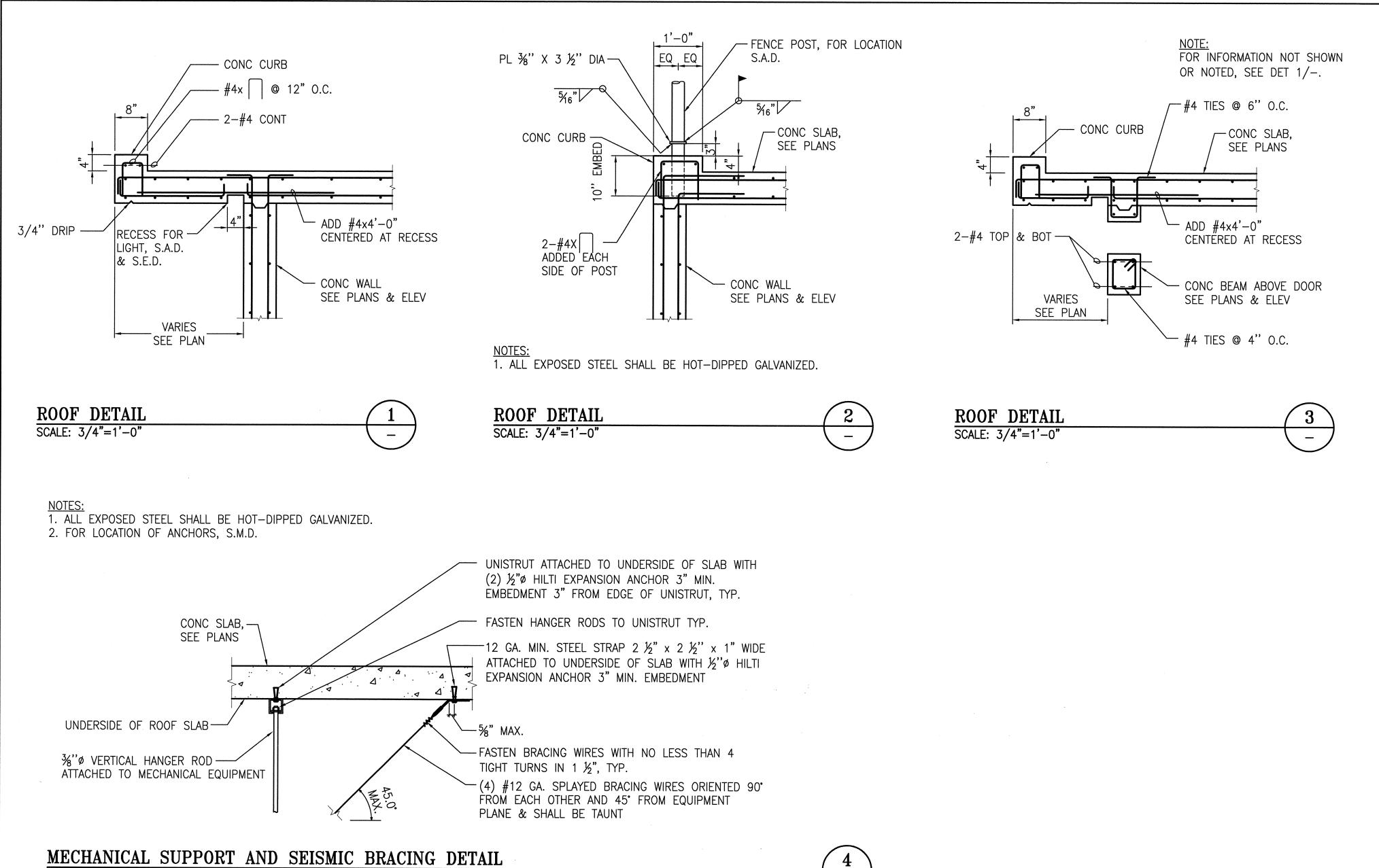
WALL SECTIONS & DETAILS

Sheet No.

S4.2

AS NOTED

Job No.



SCALE: 3/4"=1'-0"

4 -----

DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer

BUILDING DESIGN & CONSTRUCTION



Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager Suite 4100 (415) 557-4700 Fax (415) 5574701 30 Van Ness Avenue San Francisco, CA 94102-6028

Project

2008 PARK BOND RESTROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN

DUPONT TENNIS COURTS RESTROOMS 336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007

Consultant

INFRASTRUCTURE DIVISION DEPARTMENT OF PUBLIC WORKS CITY & COUNTY OF SAN FRANCISCO 30 VAN NESS AVENUE, 5TH FLOOR SAN FRANCISCO, CA 94102-6028 DESIGNED BY: 52 SL 09/2012 DRAWN BY: 52 SL 09/2012 DRAWN BY: 54 SL 09/2012 CHECKED BY: 52 JC 09/2012 APPROVED APPROVED						
DESIGNED BY: 52 SL 09/2012 DRAWN BY: BH 09/2012 CHECKED BY: J.C. JC 09/2012 APPROVED Frequence SECTION MANAGER: 0/25/12 DEPUN DIVISION MANAGER: DATE: MALL 10/26/12	DEPARTMENT OF PUBLIC WORKS CITY & COUNTY OF SAN FRANCISCO 30 VAN NESS AVENUE, 5TH FLOOR					
CHECKED BY: J.C. JC 09/2012 APPROVED Frequence 6/25/12 SECTION MANAGER: 0/25/12 DEPUN DIVISION MANAGER: DATE: COMMINSION MANAGER: DATE:	DESIGNED BY:	5V	SL			
APPROVED Frequence 6/25/12 SECTION MANAGER: 0/25/12 DEPUN DIVISION MANAGER: DATE: (all) (0/26/12	DRAWN BY:	BH	BH	09/2012		
Termin SECTION MANAGER: DEPUT DIVISION MANAGER: DATE: DEPUT DIVISION MANAGER: DATE: COMPLET 10/26/12	CHECKED BY:	d.C.	JC	09/2012		
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Cathe ispoli	SECTION MANAGE	JD.		0/25/12 0/25/12		
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NO.	Date	Revisions

Section Head . LEUNG Proj. Mgr. Proj. Arch T. LEUN Drawr Date SEPTEMBER 2012 Phase PERMIT SET

Drawing Title

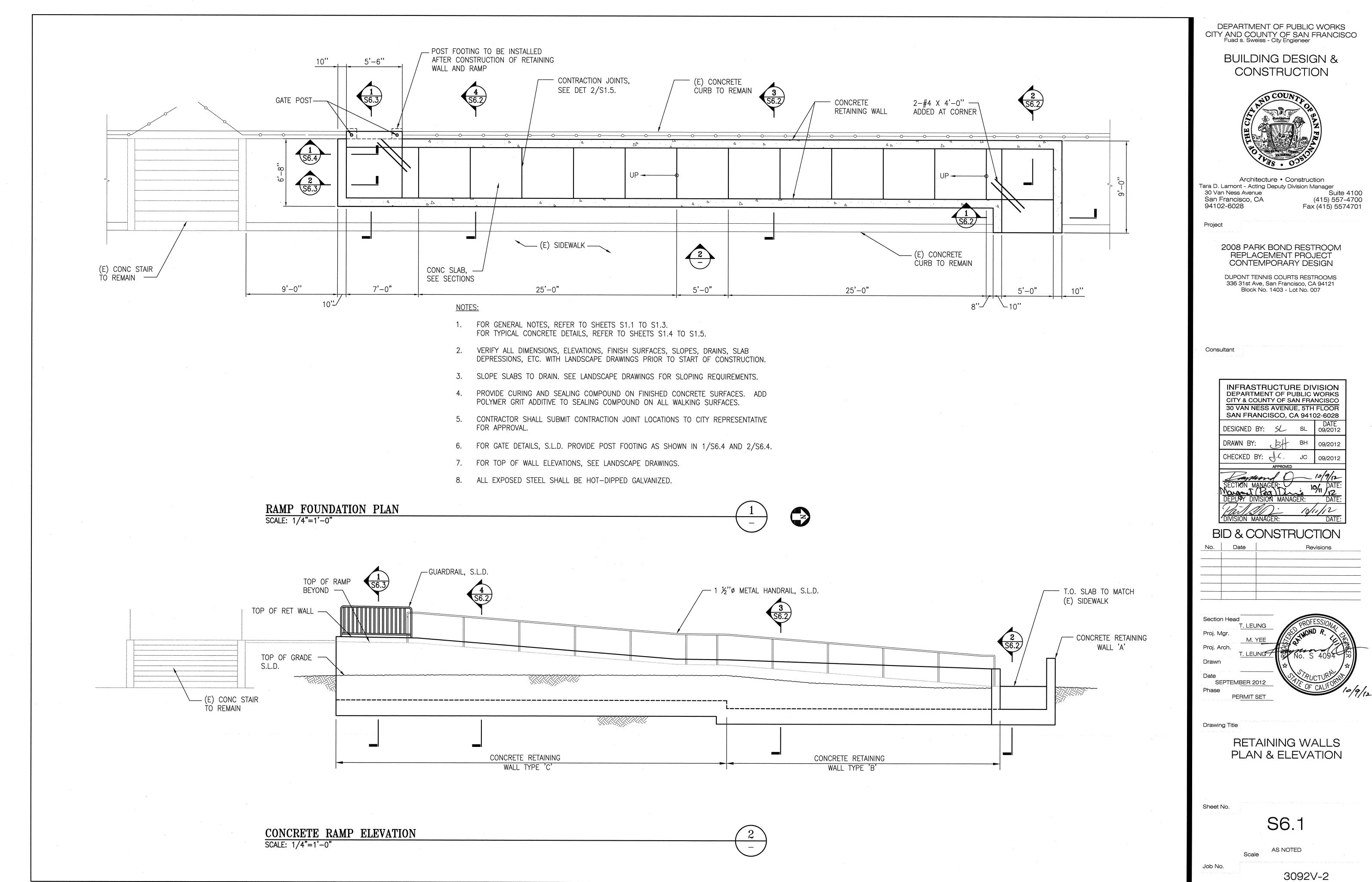
ROOF DETAILS

Sheet No.

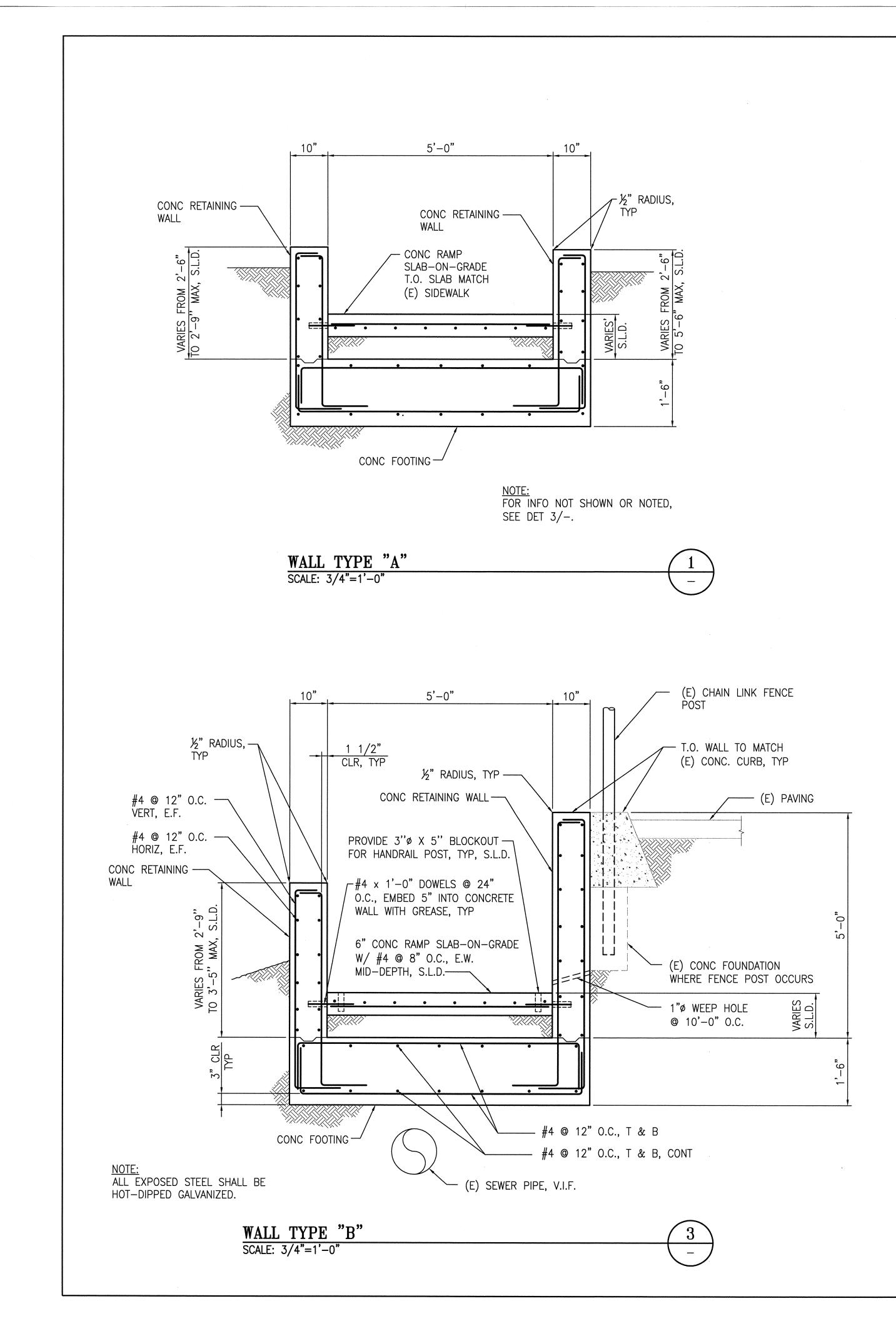
S5.1

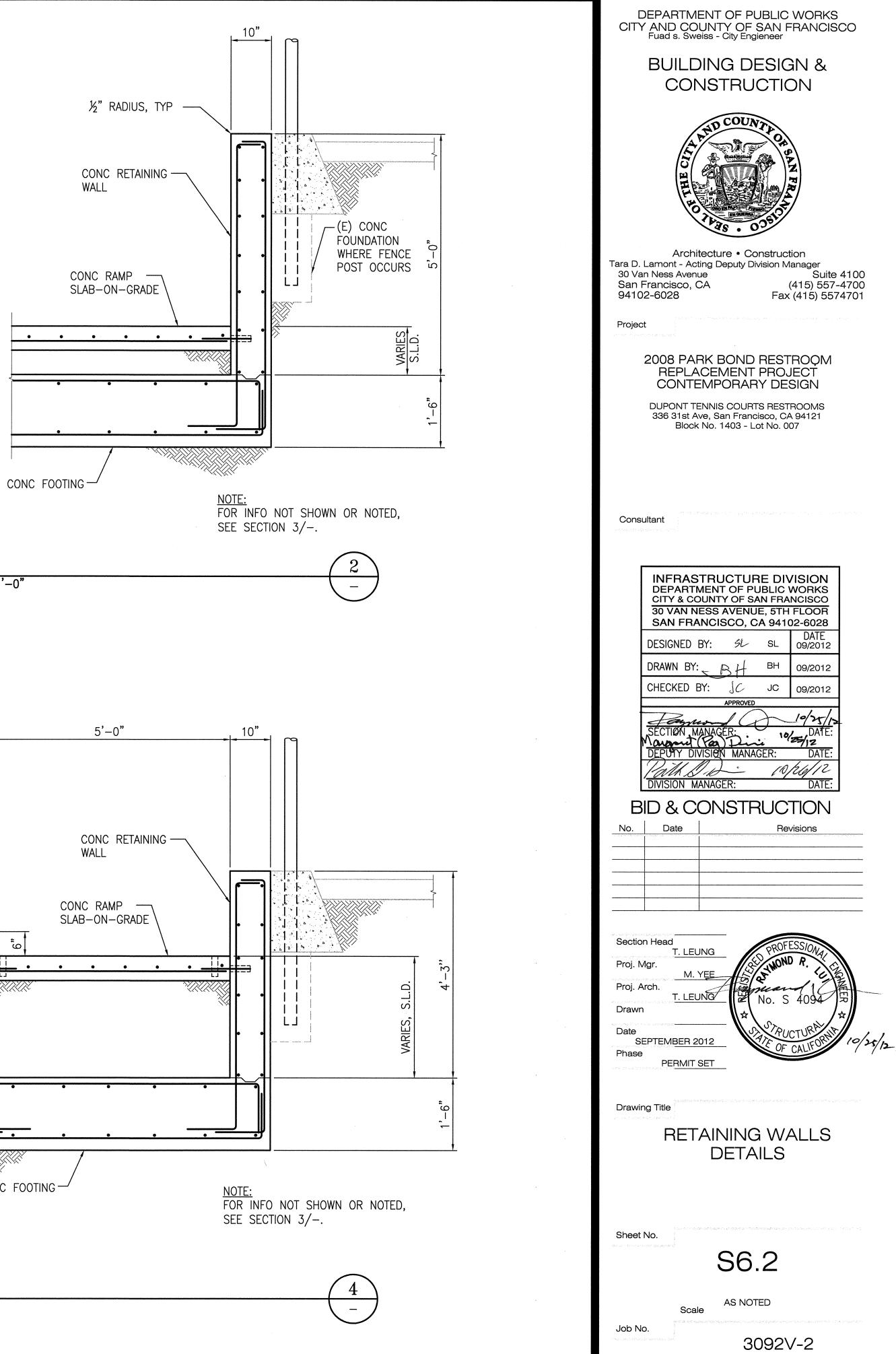
AS NOTED

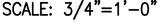
Scale

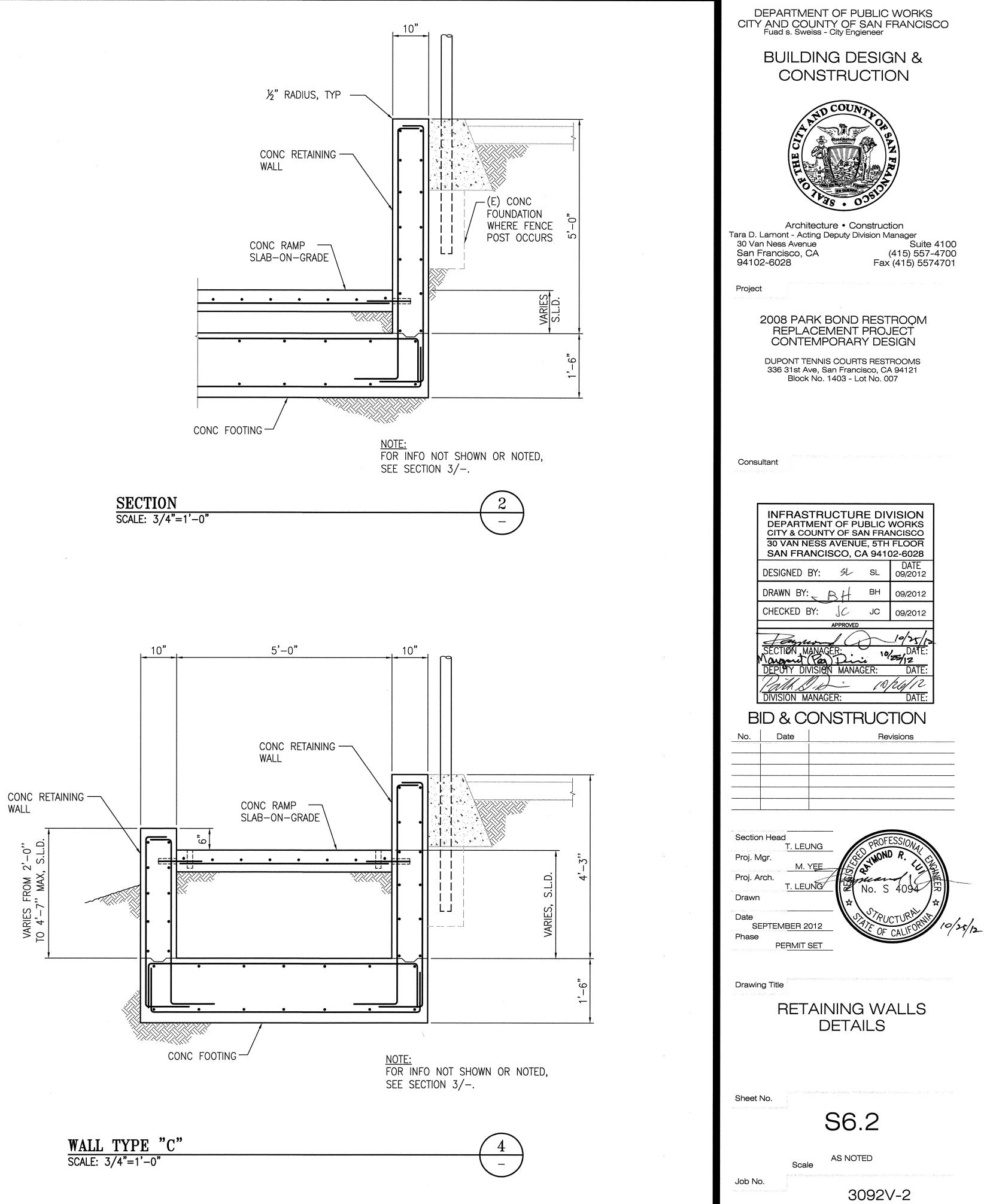


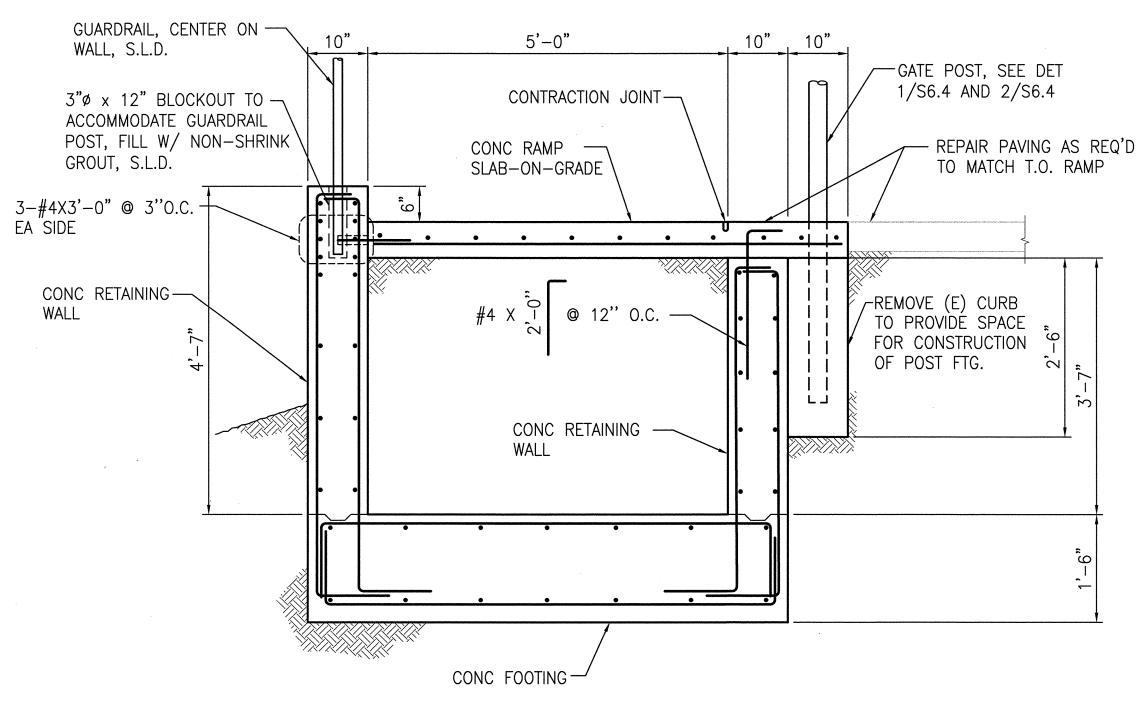
- 1 ½" Ø METAL HANDRAIL, S.L.D.	
	4
CONCRETE RETAINING WALL TYPE 'C'	







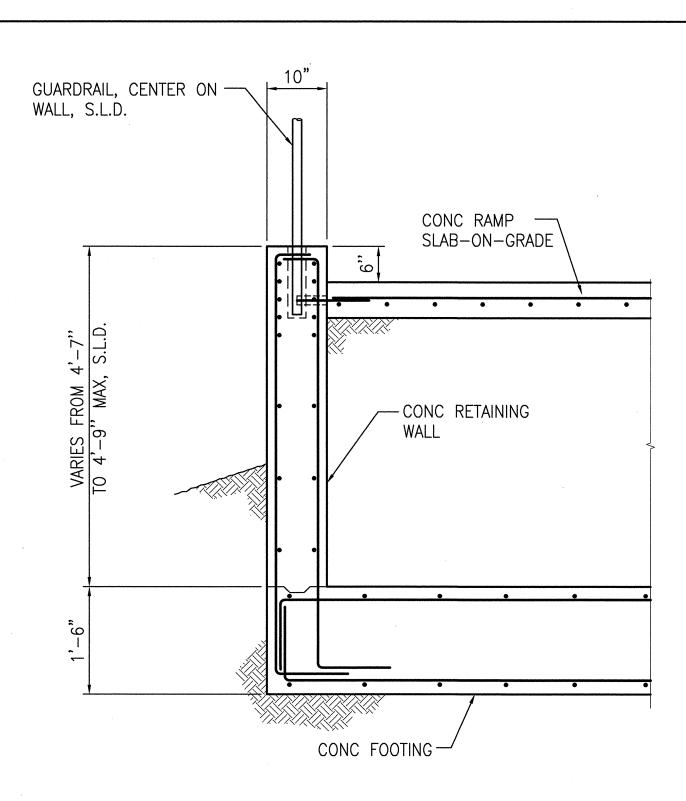




NOTE:

 $\overline{1.}$ FOR INFO NOT SHOWN OR NOTED, SEE SECTION 3/6.2. 2. ALL EXPOSED STEEL SHALL BE HOT-DIPPED GALVANIZED.

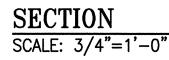
SECTION SCALE: 3/4"=1'-0"



<u>NOTE:</u>

1. FOR INFO NOT SHOWN OR NOTED, SEE SECTION 3/6.2 AND 1/-. 2. ALL EXPOSED STEEL SHALL BE HOT-DIPPED GALVANIZED.

2



DEPARTMENT OF PUBLIC WORKS CITY AND COUNTY OF SAN FRANCISCO Fuad s. Sweiss - City Engieneer

BUILDING DESIGN & CONSTRUCTION



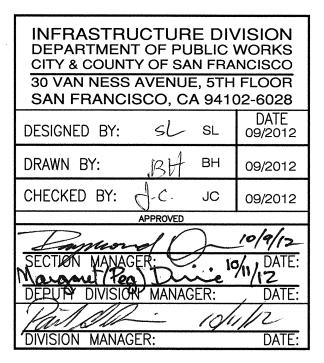
Architecture • Construction Tara D. Lamont - Acting Deputy Division Manager30 Van Ness AvenueSuite 4100San Francisco, CA(415) 557-470094102-6028Fax (415) 5574701

Project

2008 PARK BOND RESTROOM REPLACEMENT PROJECT CONTEMPORARY DESIGN

DUPONT TENNIS COURTS RESTROOMS 336 31st Ave, San Francisco, CA 94121 Block No. 1403 - Lot No. 007

Consultant



BID & CONSTRUCTION

Date	Revisions
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Section Head T. LEUNG Proj. Mgr. M. YEE	PROFESSION A
Proj. Arch. T. LEUNG Drawn	No. S 4094
Date SEPTEMBER 2012	* STRUCTURAL TO ALLEORIT
Phase PERMIT SET	OF CALITY TY IS

Drawing Title

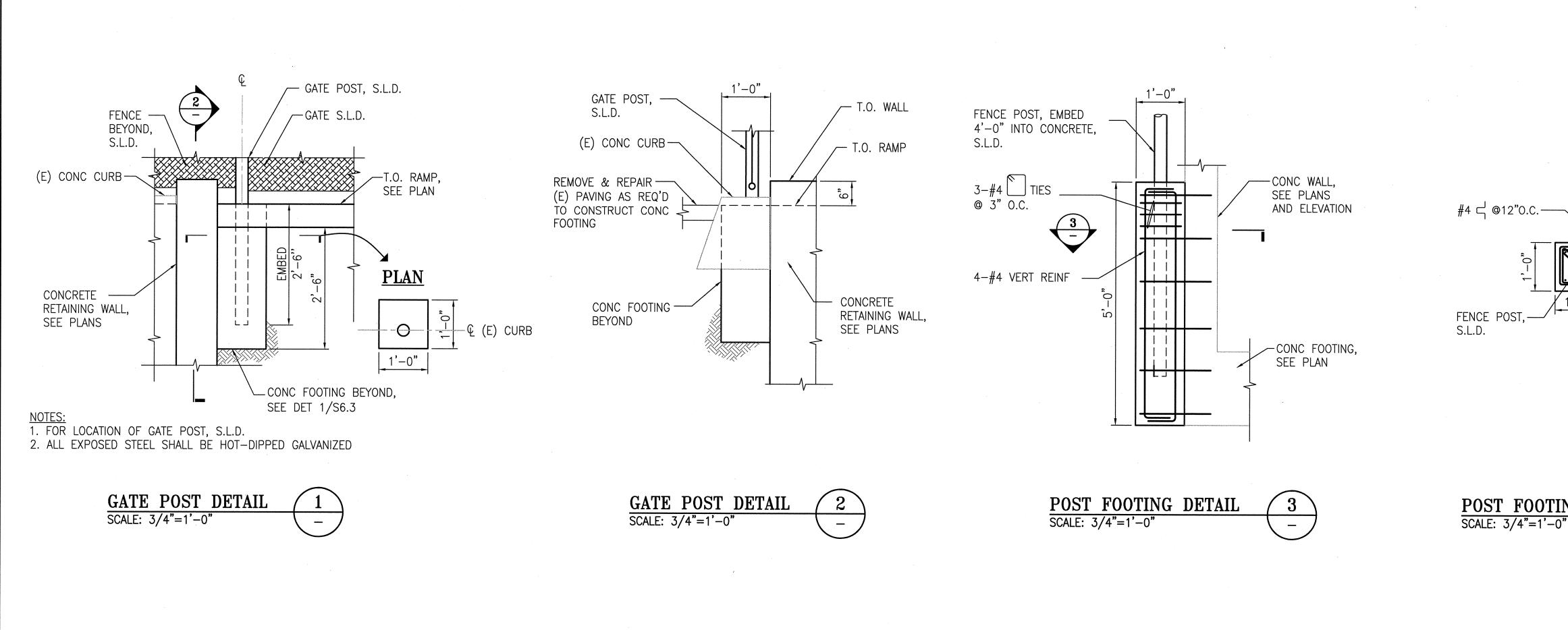
RETAINING WALLS DETAILS

Sheet No.

S6.3

AS NOTED

Scale Job No.



1

